



**COMMITTEE OF ADJUSTMENT NOTICE OF PUBLIC HEARING  
APPLICATION NO. A-012-2024**

**TAKE NOTICE** that an application has been received by the Town of Innisfil from **Leanna Mohammed, applicant**, on behalf of **Agostino Dimarsico, Owner**, for a minor variance from Zoning By-law 080-13, pursuant to Section 45 of the *Planning Act*, R.S.O. 1990, c. P.13, as amended.

The subject property is described legally as **PLAN 767 LOT 55** and is known municipally as **1041 Fern Road** and is zoned as **“Residential 1 (R1)”**.

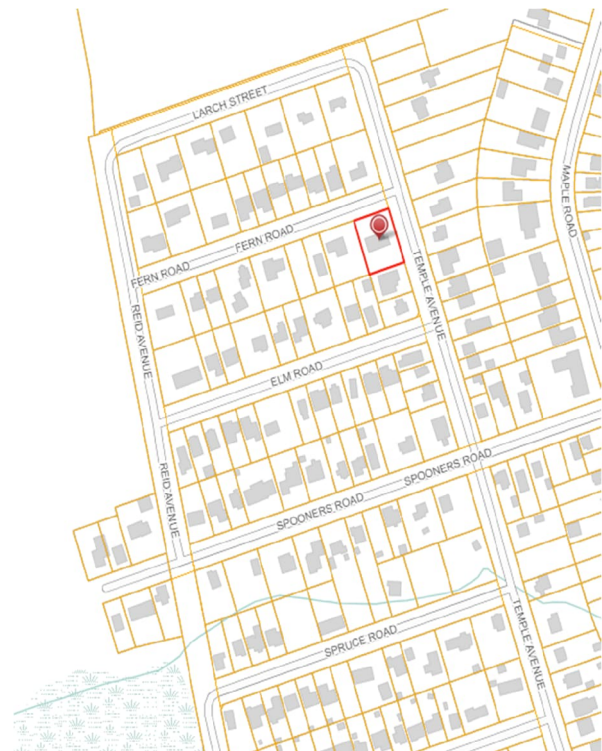
**The applicant is proposing to construct a detached garage with a proposed Gross Floor Area (GFA) of 117.05 m<sup>2</sup>. The applicant is seeking relief from Section 3.3(a) of the Zoning By-law permits a maximum total lot coverage of 10% for accessory structures in residential zones.**

The Committee of Adjustment for the Town of Innisfil will consider this application in person at Town Hall and virtually through Zoom on **Thursday, May 16, 2024, at 6:30 PM**.

To participate in the hearing and/or provide comments, you must register by following the link below or scanning the above QR code: <https://innisfil.ca/en/building-and-development/committee-of-adjustment-hearings.aspx>

Requests can also be submitted in writing to: Town of Innisfil Committee of Adjustment, 2101 Innisfil Beach Road, Innisfil, Ontario, L9S 1A1 or by email to [planning@innisfil.ca](mailto:planning@innisfil.ca).

If you wish to receive a copy of the decision of the Committee of Adjustment in respect of the proposed minor variance, you must make a written request to the Secretary-Treasurer of the Committee of Adjustment by way of email or regular mail. The Notice of Decision will also explain the process for appealing a decision to the Ontario Lands Tribunal.



Additional information relating to the proposed application is available on the Town of Innisfil website. Accessible formats are available on request, to support participation in all aspects of the feedback process. To request an alternate format please contact Planning Services at [planning@innisfil.ca](mailto:planning@innisfil.ca).

Dated: **May 1, 2024**

Toomaj Haghshenas,  
Secretary-Treasurer  
[thaghsheenas@innisfil.ca](mailto:thaghsheenas@innisfil.ca)  
705-436-3710 ext. 3316

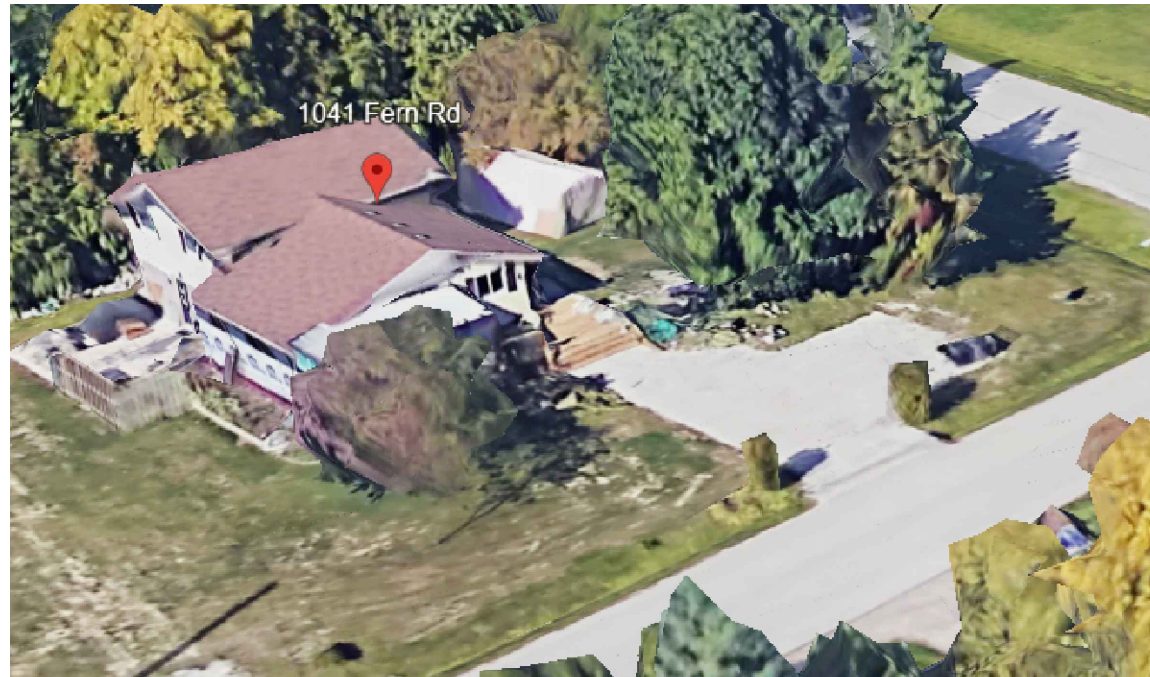
# Agustino Dimarsico

1041 Fern Avenue, Innisfil, ON,  
L9S 4R7

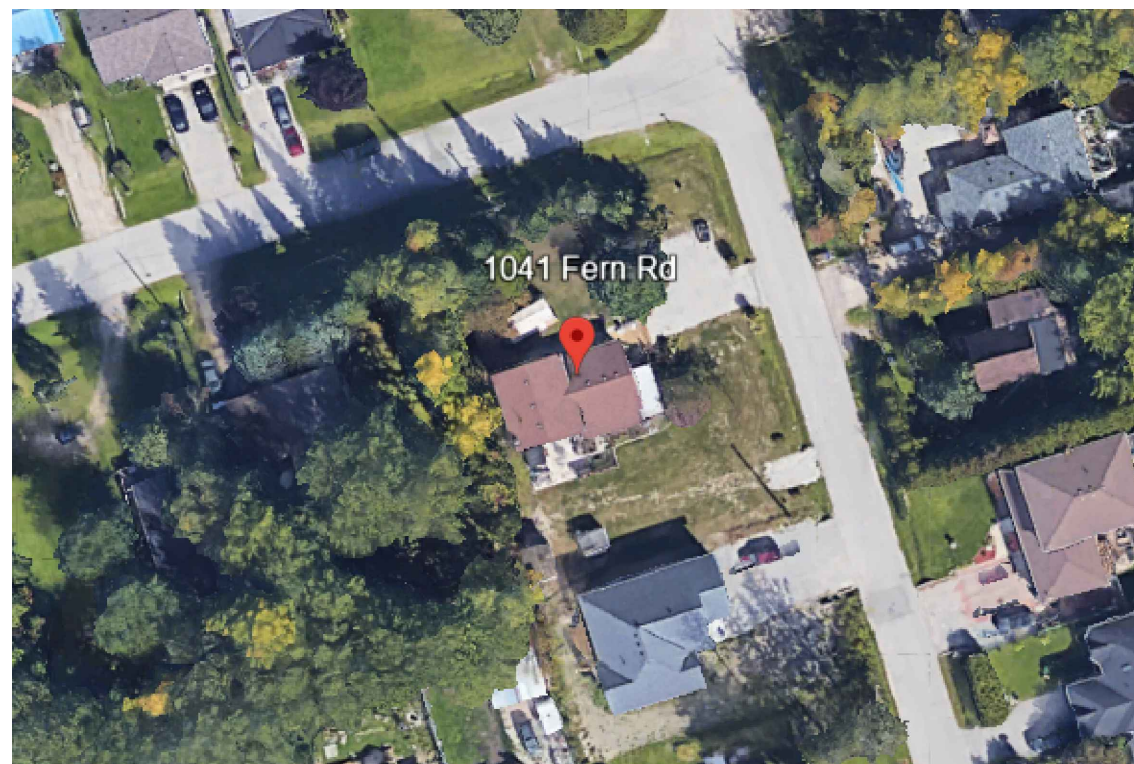
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**3D**



**SITE PLAN**



Rev.	Date	By	Description
01	04-26-24	GT	City setback by-law

CLIENT:  
**Agustino Dimarsico**

PROJECT NAME  
**Agustino Dimarsico**

PROJECT LOCATION  
1041 Fern Avenue, Innisfil,  
ON, L9S 4R7

DRAWING TITLE:  
**TITLE PAGE**

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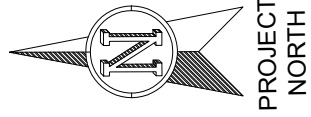
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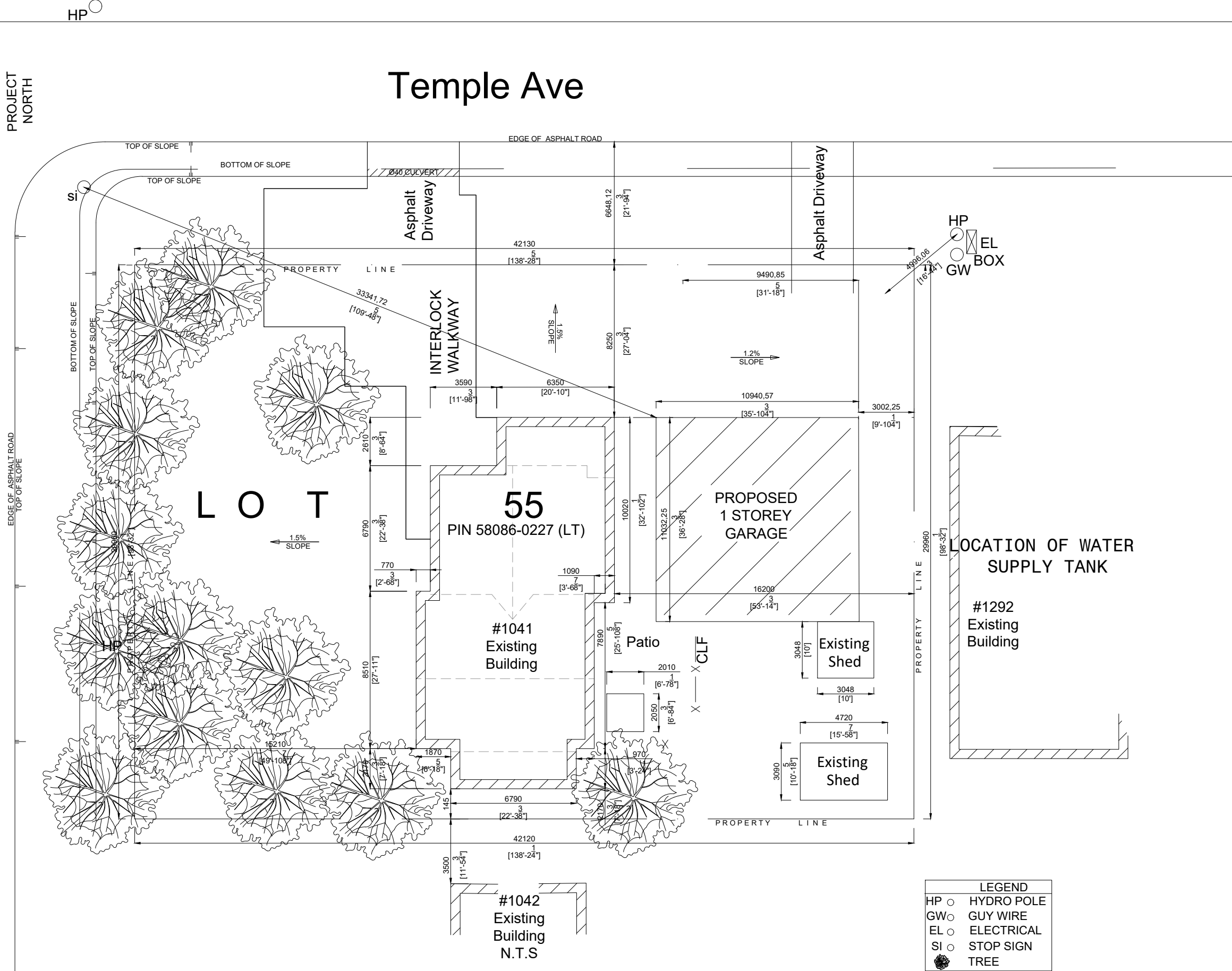
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# Temple Ave

# Fern Rd



LEGEND	
HP ○	HYDRO POLE
GW ○	GUY WIRE
EL ○	ELECTRICAL
SI ○	STOP SIGN
	TREE

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PROJECT LOCATION  
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DRAWING TITLE:  
**SITE PLAN**

SCALE: <b>N.T.S</b>	DRAWN BY: <b>M.S</b>	CHECKED BY:
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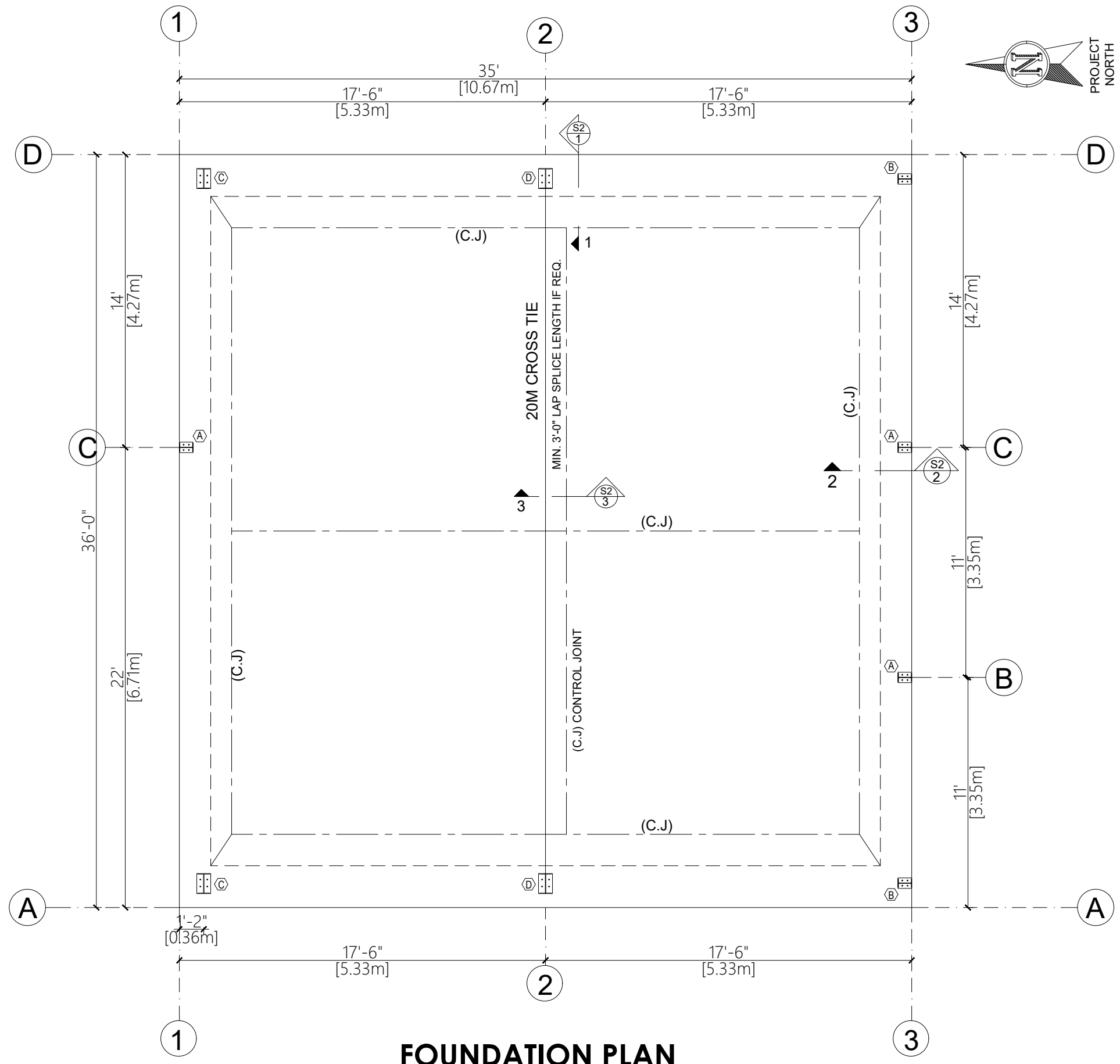


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NOTE:



**FOUNDATION PLAN**

01	04-26-24	GT	City setback by-law
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DRAWING TITLE:  
**FOUNDATION PLAN**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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DRAWING #: <b>A-200</b>	DATE: <b>APR. 2024</b>
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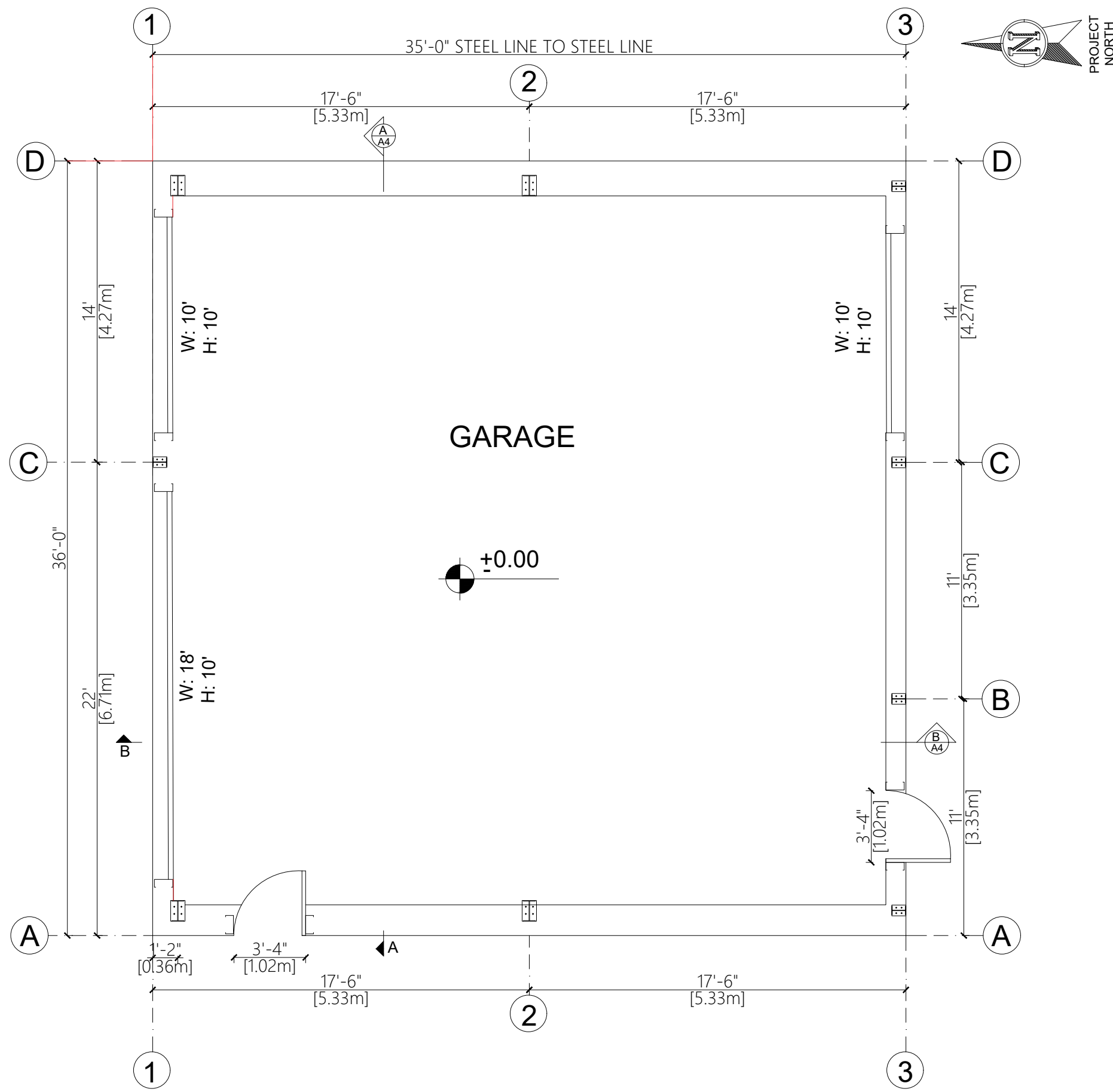
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DRAWING TITLE:  
**FLOOR PLAN**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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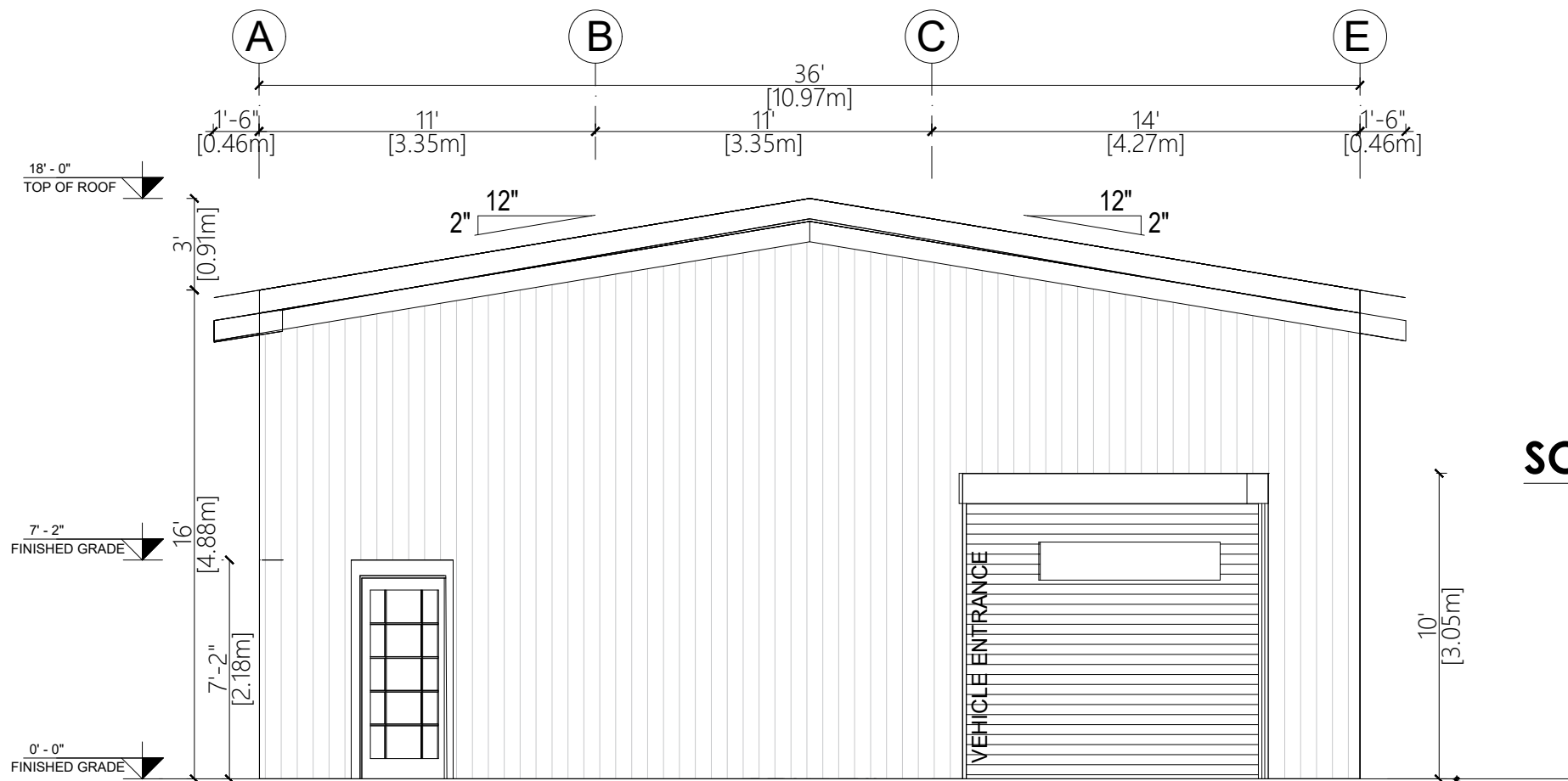
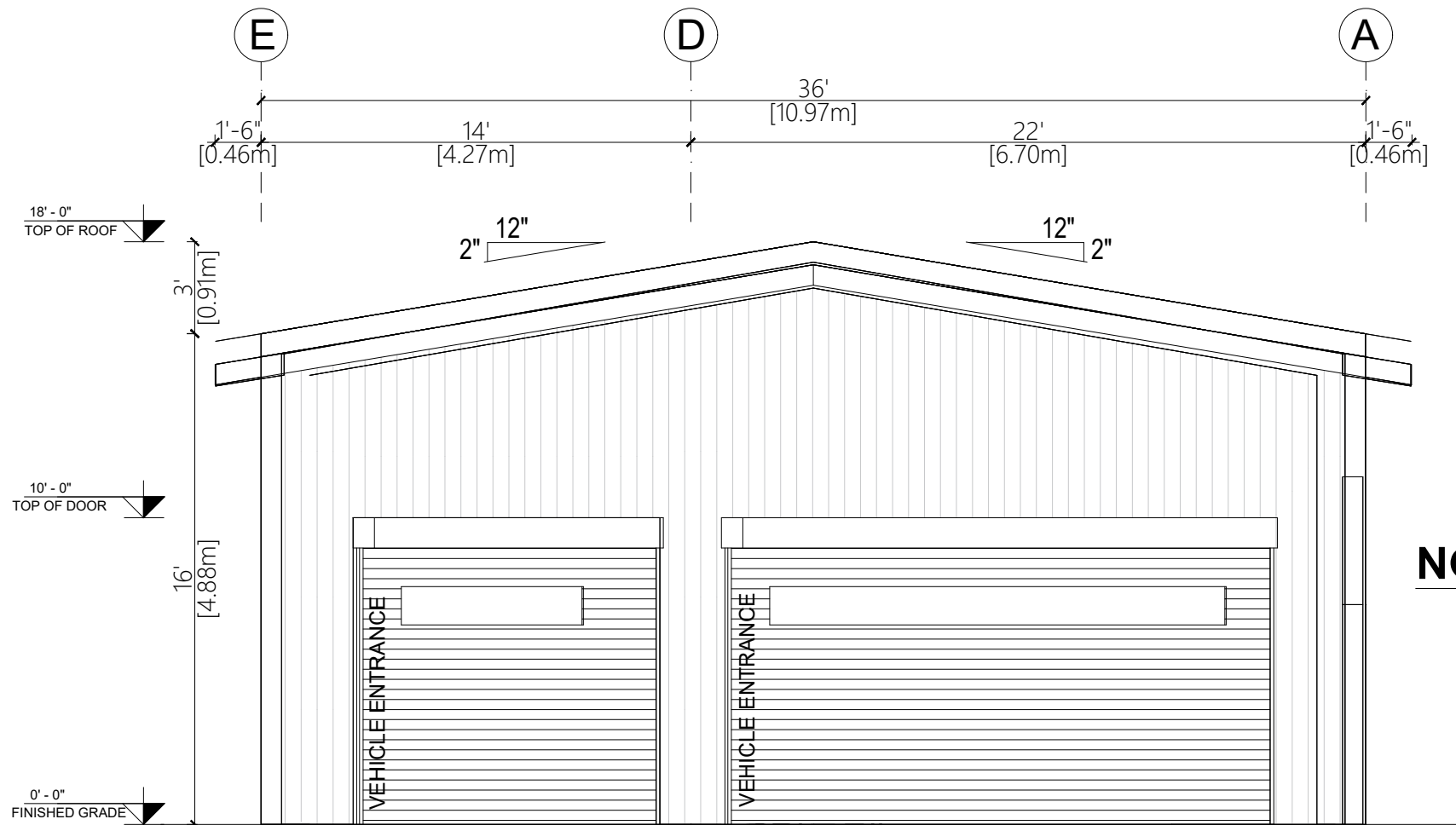
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DRAWING TITLE:  
**ELEVATIONS**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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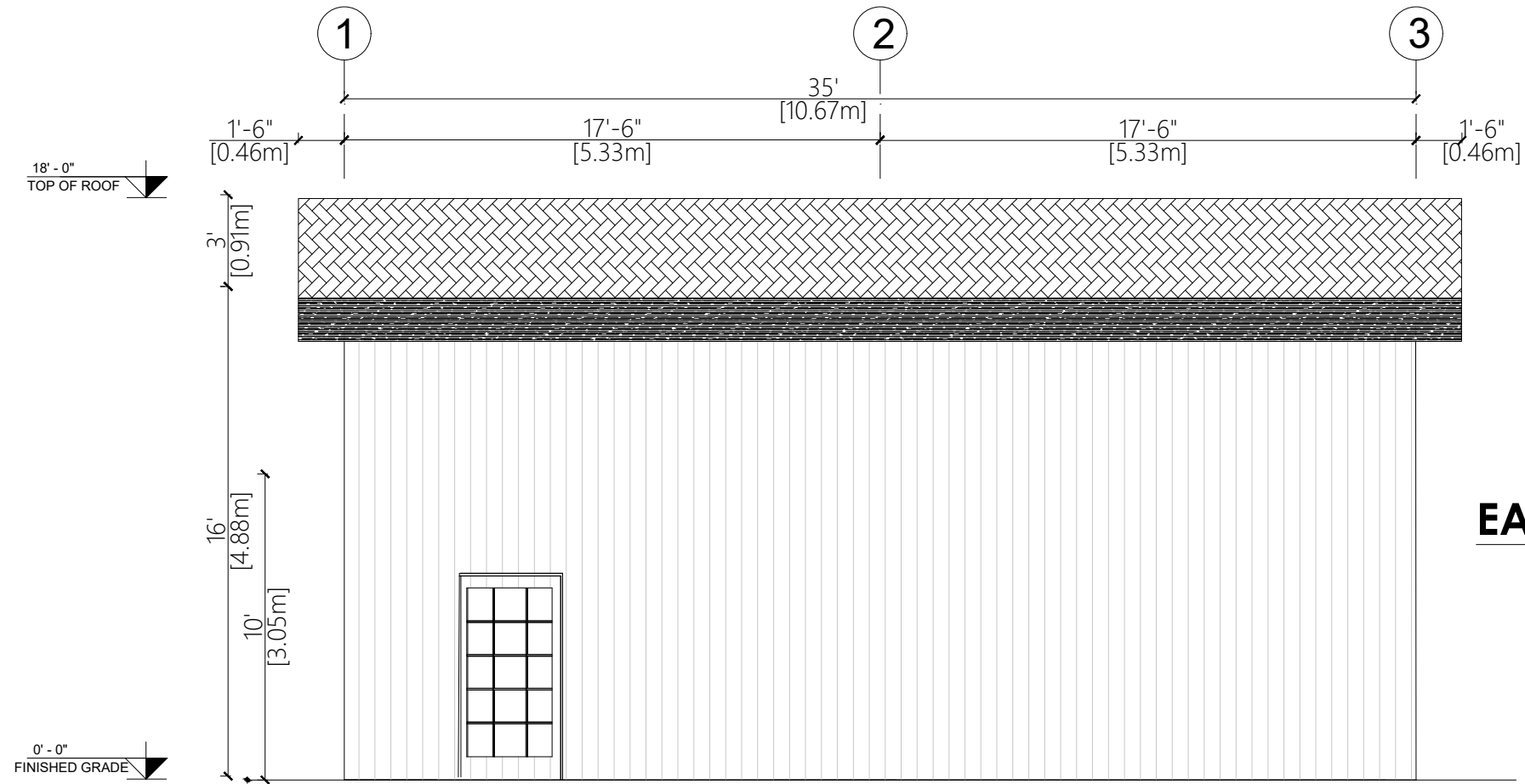
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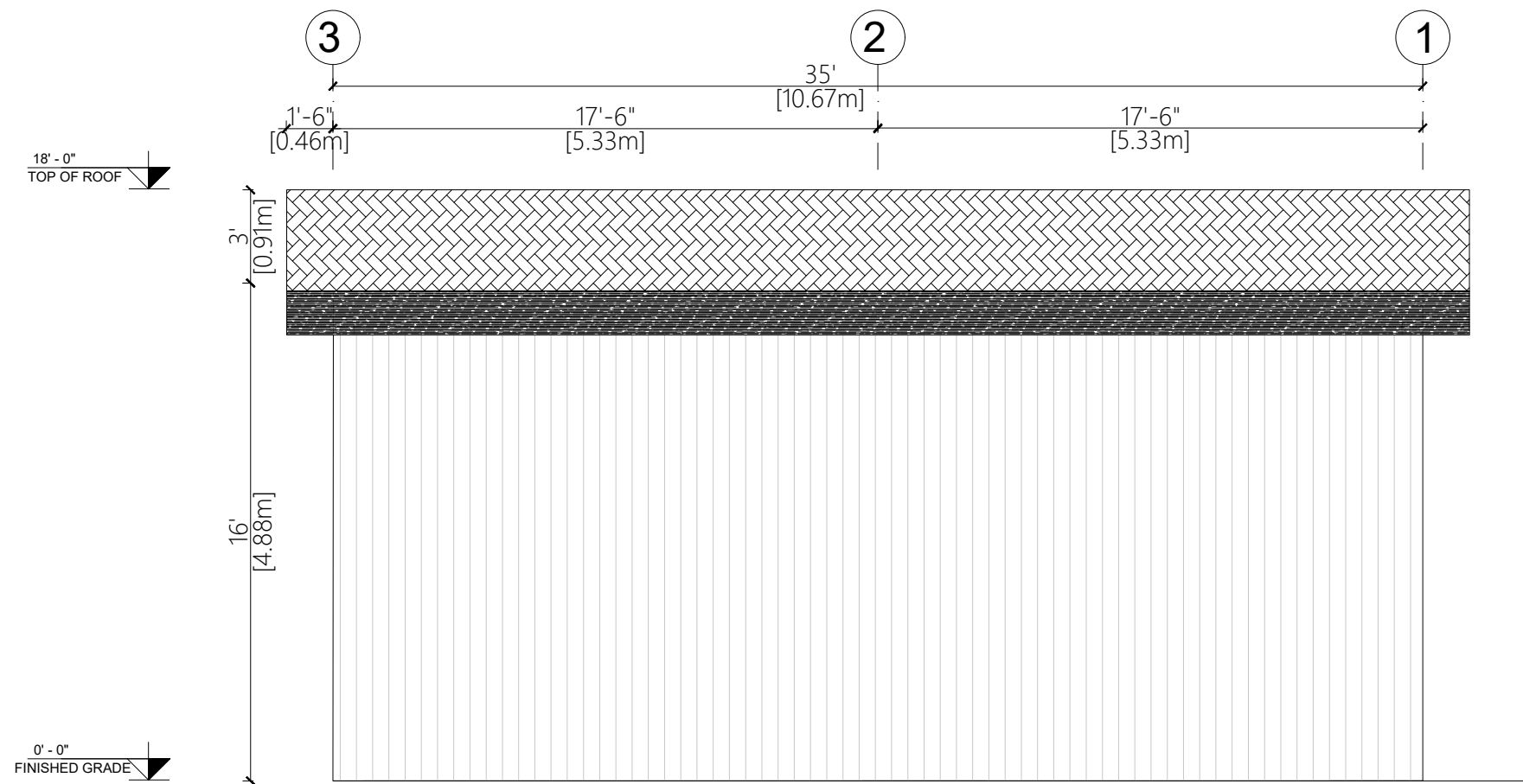
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**EAST ELEVATION**



**WEST ELEVATION**

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DRAWING TITLE:  
**ELEVATIONS**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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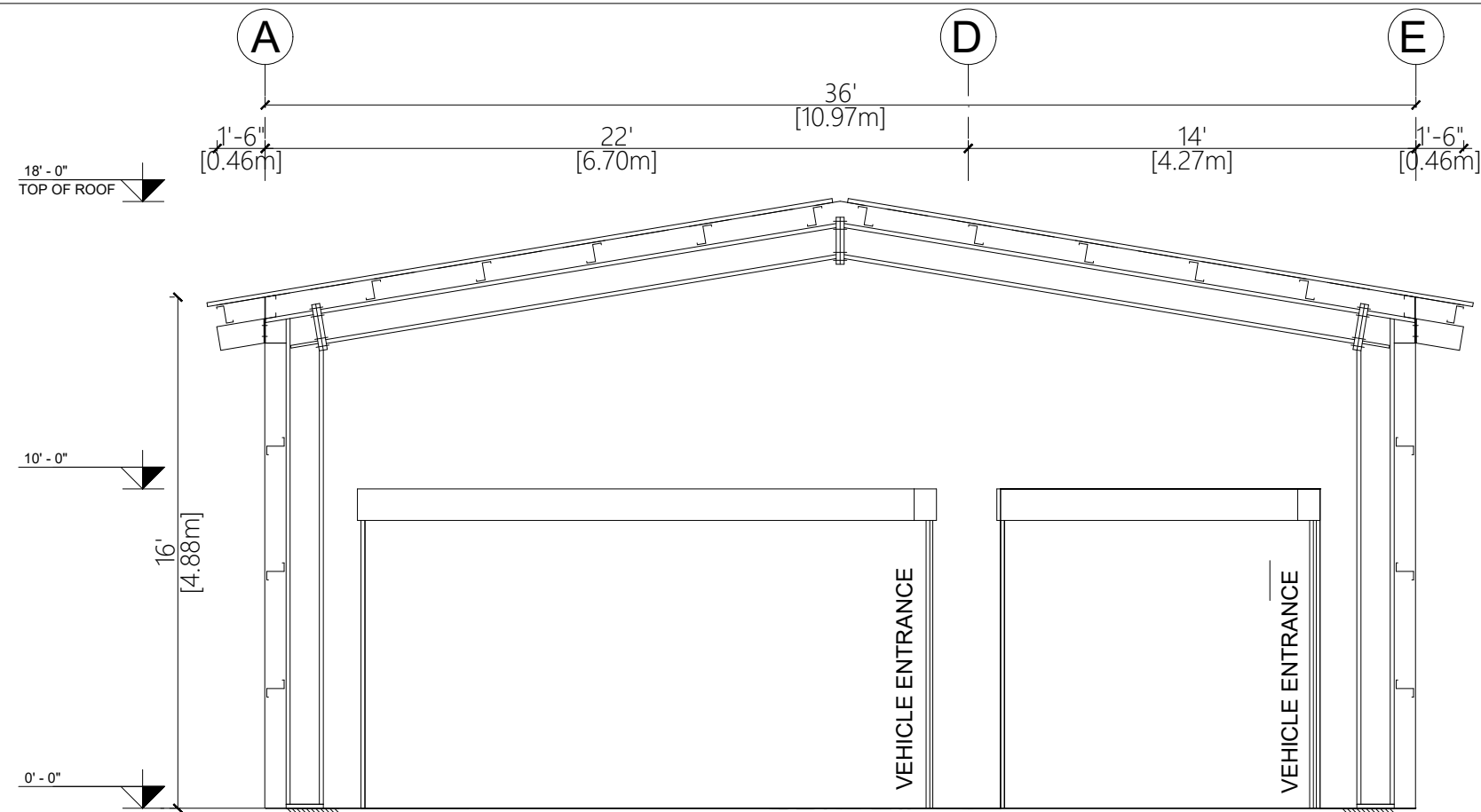
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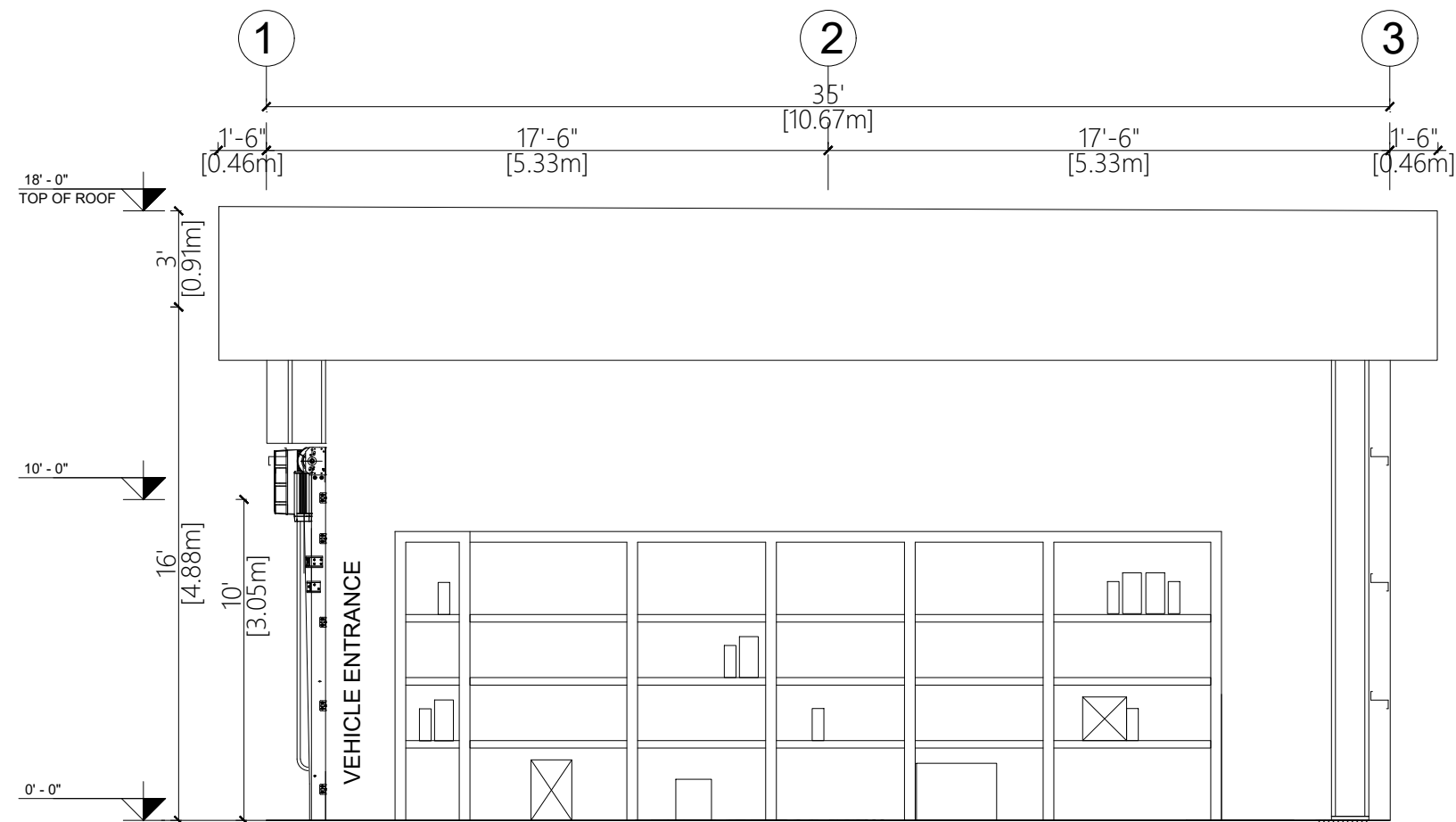


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**SECTION A-A**



**SECTION B-B**

Rev.	Date	By	Description
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**Agustino Dimarsico**

PROJECT LOCATION  
1041 Fern Avenue, Innisfil,  
ON, L9S 4R7

DRAWING TITLE:  
**SECTION PLAN**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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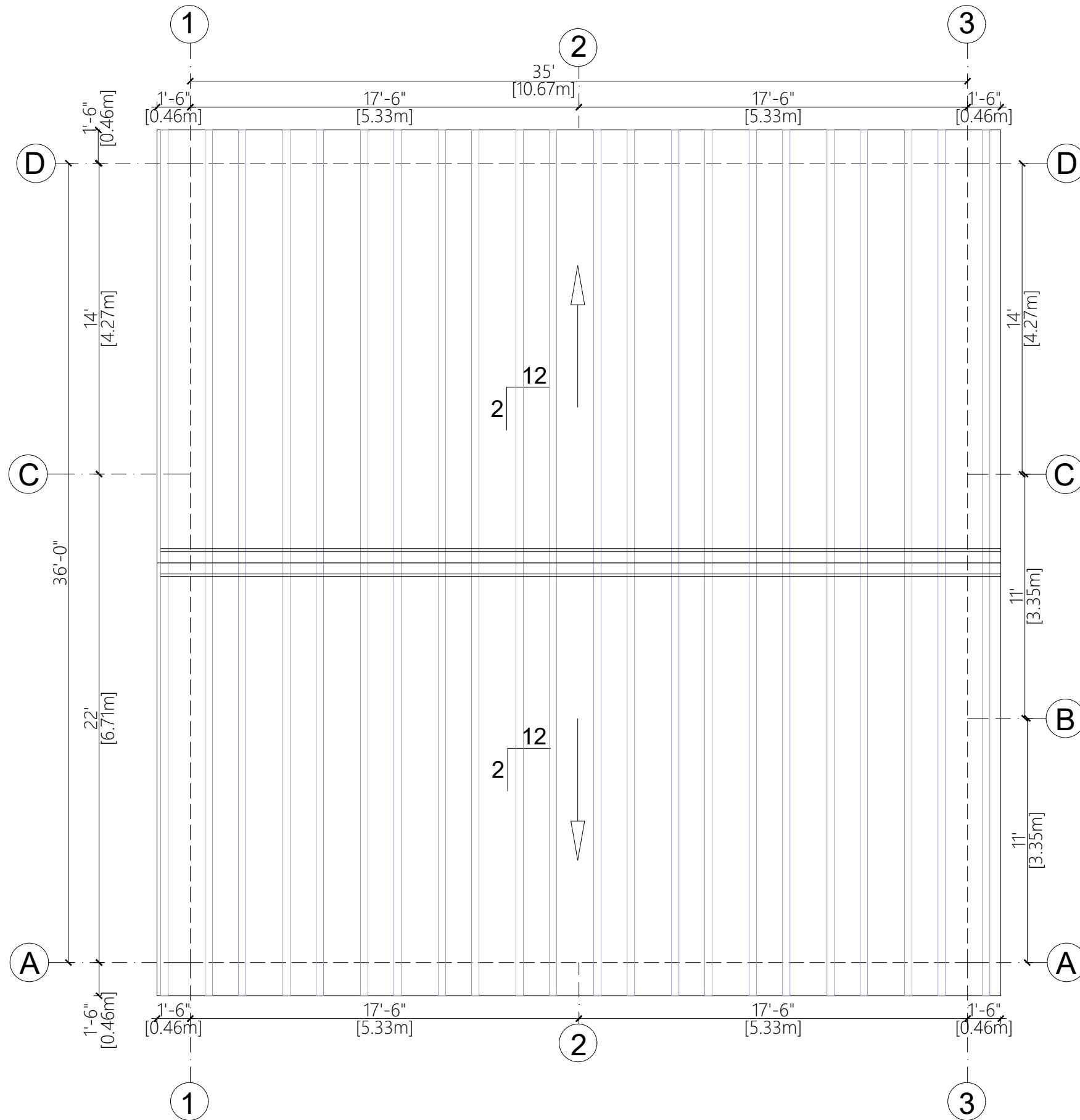
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**ROOF PLAN**

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**Agustino Dimarsico**

PROJECT LOCATION  
1041 Fern Avenue, Innisfil,  
ON, L9S 4R7

DRAWING TITLE:  
**ROOF PLAN**

SCALE: 3/16"=1'-0"	DRAWN BY: <b>M.S</b>	CHECKED BY: <b>B.T</b>
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DRAWING #: <b>A-500</b>	DATE: <b>APR.2024</b>
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**GENERAL NOTES**

- ALL DRAWINGS TO WHICH THEY RELATE CONTAIN THESE NOTES. IF THERE ARE DISCREPANCIES EXIST BETWEEN DRAWINGS, NOTES OR THE CODES, THE MOST RESTRICTIVE SHALL APPLY.
- DRAWINGS ARE NOT BE SCALED.
- ALL STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH APPLICABLE ARCHITECTURAL, MECHANICAL, ELECTRICAL, LANDSCAPING, ETC. DRAWINGS.
- THE ENGINEER MUST BE NOTIFIED OF ANY DISCREPANCIES FOUND IN THE DRAWINGS BEFORE BEGINNING ANY CONSTRUCTION OR PRELIMINARY WORK.
- THE PROVINCIAL OCCUPATIONAL HEALTH AND SAFETY ACT, REGULATIONS, ALL APPLICABLE CODES, ORDINANCES, AND RECOGNIZED INDUSTRY STANDARDS MUST ALL BE FOLLOWED IN THE COURSE OF WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE CURRENT CONDITIONS AND ENSURING ALL NEW WORK IS COMPATIBLE WITH THE EXISTING CONDITIONS.
- TO AVOID DAMAGE TO THE EXISTING STRUCTURE, THE CONTRACT SHALL TAKE ALL PREVENTIVE MEASURES. IF THE CONTRACTORS CAUSE DAMAGE TO THE EXISTING STRUCTURE, DELCOR ENGINEERING INC. WILL NOT BE HELD LIABLE.
- PRIOR TO WORK COMMENCING, IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL UTILITY SERVICES WITHIN THE WORK AREA. THE CONTRACTOR MUST CONTACT THE OWNERS REPRESENTATIVE AT MINIMUM 48 HOURS IN ADVANCE OF ANY EVENTS THAT CAN CAUSE DISRUPTIONS WHILE CONSTRUCTION IS UNDERWAY.
- THE CONTRACTOR IS REQUIRED TO NOTIFY THE ENGINEER AT LEAST 48 HOURS IN ADVANCED FOR ENGINEERING TEST SERVICES SUCH AS CONCRETE AND COMPACTION.
- THE SPECIFICATIONS, ENGINEERING, DESIGN AND PLANS THAT ARE PROVIDED ARE ONLY FOR THE PROJECT THAT IS MENTIONED IN THIS DOCUMENT. IF ANY OF THESE PLANS, SPECIFICATIONS OR THE ACCOMPANYING ADVICE, DESIGN, OR INSTRUCTIONS ARE USED ON ANY PROJECT OR AT A LOCATION OTHER THAN MENTIONED ABOVE, DELCOR ENGINEERING INC. DISCLAIMS ALL LIABILITY.
- BEFORE STARTING WORK, THE CONTRACTOR SHALL CONFIRM ALL DIMENSIONS AND CONDITIONS ON THE PROJECT SITE AND NOTIFY THE ENGINEER ANY ERRORS, OMISSIONS, OR POSSIBLE DISCREPANCIES BETWEEN FIELD CONDITIONS AND DRAWINGS. THE SITE AND BUILDING LAYOUT SHALL RECEIVE SPECIAL CARE.

**FOUNDATION AND GEOTECHNICAL NOTES**

- A MINIMUM OF 200mm (6") GRANULAR MATERIAL COMPACTED TO 98% S.P.M.D.D UNDER ALL SLABS.
- THE FOUNDATIONS HAVE BEEN DESIGNED TO SUPPORT A MIN. NET BEARING CAPACITY OF SLS 150kPa (3133 psf) /ULS (FACTORED) 225kPa (4700 psf) AND AN ASSUMED FACTORED SLIDING COEFFICIENT OF 0.55 FOUND ON NATURAL UNDISTURBED INORGANIC SOIL.
- AN ON-SITE GEOTECHNICAL ENGINEER MUST APPROVE THE ASSIGNED SAFE NET BEARING PRESSURE FOR EACH FOOTING AS WELL AS THE MINIMUM SITE CLASS 'D' AS PER OBC 4.1.8.4. THE FOUNDATION DETAILS WILL BE MODIFIED BY THE ENGINEER IN ACCORDANCE WITH THE CURRENT SITE CONDITIONS IN THE EVENT THAT THE NET BEARING PRESSURE OR SITE CLASSIFICATION USED FOR DESIGN IS NOT APPROVED. ALL FOOTINGS SHALL BE CONSTRUCTED ON SUITABLE BEARING MATERIAL.
- SOFT OR FROZEN SOIL MATERIAL BENEATH FOUNDATIONS CAN BE REPLACED WITH CONCRETE OF 15 MPa (2000psi) AND HAS A MINIMUM 28 DAY STRENGTH TO THE UNDERSIDE OF FOOTING.
- THE CONTRACTOR MUST CONSTRUCT THE FOOTING ON A LEVEL SKIM SLAB OF 2" THICK AND 15 MPa (2000psi) MINIMUM 28 DAY STRENGTH, IF SOIL SOFTENING OCCURS AFTER EXCAVATION OR AS REQUIRED BY THE SOILS REPORT. THIS SHALL BE DONE PRIOR TO ANY SUBSEQUENT CONCRETE FOUNDATION POUR.
- EXTERIOR WALLS AND COLUMN FOOTINGS ARE TO BE INSTALLED AT LEAST 4'-0" (ASSUMED FROST DEPTH) BELOW THE FINISHED GRADE. THE ASSUMED FROST DEPTH WILL BE CONFIRMED BY A GEOTECHNICAL ENGINEER PRIOR TO INSTALLATION. DURING CONSTRUCTION, ALL FOOTINGS, WALLS, SLABS ON-GRADE , AND ADJACENT SOIL WILL BE PROTECTED FROM FREEZING AND FROST ACTION (DOES NOT APPLY TO FLOATING SLABS).
- IF THERE ARE VARIATIONS OF THE FOOTING SUBSIDE ELEVATION, STEP FOOTINGS ARE REQUIRED. THE DETAILS ARE TO BE SPECIFIED IN THE FOUNDATION PLAN.
- FOOTING ELEVATIONS SHALL BE LOCATED FOR BURIED ELECTRICAL OR MECHANICAL SERVICE WITHIN THE SITE. THE SLOPE OF THE LINE BETWEEN ADJACENT FOOTING ELEVATIONS TO 7 IN 10 MAXIMUM WITH A MAXIMUM RISE OF 2'-0" IS USED TO PREVENT UNDERMINING AND OVERLOADING OF ADJACENT AND EXISTING FOOTINGS,
- ASTM 820 TYPE 1 MUST BE COMPLIANT WHEN USING STEEL FIBRES FOR REINFORCE CONCRETE. IT MUST BE PLACED AND PREPARED IN COMPLIANCE WITH ASTM C1116 REQUIREMENTS.
- NON-METALIC REINF. SHALL BE COMPLIANT WITH EUROPEAN STANDARD EN 14889-22006 FIBRES FOR CONCRETE PART 2: CLASS 1A AND CARRIES CE MARKING. ASTM C1116 REQUIREMENTS REGARDING PREPARATION AND PLACEMENT MUST BE FOLLOWED FOR FIBRE-REINF. CONCRETE.

- EXCAVATED MATERIAL TO BE RE-USED AS BACKFILL MUST BE APPROVED BY THE GEOTECHNICAL CONSULTANT. BACKFILL UNDERSIDE OF SLAB-ON-GRADE WITH OPSS GRANULAR 'B' MUST BE COMPACTED IN 6" MAXIMUM LIFTS TO 98% STANDARD PROCTOR DENSITY AT OPTIMUM MOISTURE CONTENT.
- SLAB-ON-GRADE MAY BE PLACED ON NON-ORGANIC MATERIAL FILL SOIL. ALL FILL MATERIAL MUST BE APPROVED BY THE SOILS ENGINEER. IF THE FILL IS FOUND NOT ACCEPTABLE, REMOVE IT TO UNDISTURBED SOIL AND REPLACE IT WITH GRANULAR 'B.'
- PLACING BACKFILL ON BOTH SIDES OF THE WALL MUST BE DONE SIMULTANEOUSLY, AND AT NO POINT IN TIME THE HEIGHT DIFFERENTIAL BETWEEN THE TWO SIDE EXCEEDIES 2'-0". BACKFILL IS NOT PERMITTED AGAINST NEWLY POURED CANTILEVERED RETAINING WALLS UNTIL THE CONCRETE HAS ACHIEVED THE DESIGN 28 DAY STRENGTH.
- THE SLAB-ON-GRADE MAY HEAVE RESULTING IN ADDITIONAL SLAB CRACKING DUE TO THERMAL EFFECTS. THIS HEAVING IS CAUSED BY THE PERIMETER RIGID INSULATION NOT BEING INSTALLED AND THE NATURAL SOIL BASE IS NOT FREE DRAINING GRANULAR MATERIAL, AND/OR THE WATER TABLE IS NOT BELOW FROST DEPTH. AS SUCH, THIS WILL NOT BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- ARCHITECTURAL DRAWINGS MUST BE USED TO VERIFY ALL FOOTING LOCATIONS AND DIMENSIONS. UNLESS OTHERWISE SPECIFIED ON THE STRUCTURAL DRAWINGS, HORIZONTAL CONSTRUCTION OR "COLD JOINTS" IN CONCRETE RETAINING WALLS ARE NOT PERMITTED.
- THE FIRST FLOOR SLAB MUST BE CONSTRUCTED AND THE CONCRETE 28 DAY DESIGN STRENGTH FOR BOTH THE WALLS AND THE FLOOR HAS BEEN ATTAINED BEFORE BACKFILL CAN BE PLACED AGAINST THE BASEMENT RETAINING WALLS
- THE DESIGN OF RETAINING EARTH WALLS HAS BEEN DONE WITH AN ASSUMPTION OF A 15 kPa (250psf) SURCHARGE. THE LATERAL PRESSURE OF SOIL AGAINST RETAINING WALLS HAS BEEN ASSUMED TO BE 38pcf EQUIVALENT FLUID PRESSURE (ASSUMED DRAINED MATERIAL).

**STEEL AND CONCRETE CONSTRUCTION**

- STEEL BARS INTENDED FOR REINFORCEMENTS MUST BE MANUFACTURED IN ACCORDANCE WITH CSA STANDARD CSA-G30.18 AND GRADE 400 (400 MPa / 60,000psi).
- ASTM STANDARD A1064/A1064M MSUT BE COMPLIANT FOR WELDING WIRE FABRIC, WHERE THE 450 MPa (65,000 psi) IS THE MINIMUM YIELD STRENGTH.
- THE RECENT ISSUE OF THE REINFORCING STEEL MANUAL OF STANDARD PRACTICE PUBLISHED BY THE REINFORCING STEEL INSTITUTE OF CANADA HAS BEEN USED TO DETAIL STANDARDS FOR ALL REINFORCING STEEL. UNLESS OTHERWISE NOTED, LAP CONTINUOUS REINF. STEEL 30 BAR DIAMETERS AT SPLICES AND CORNERS, WITH THE EXCEPTION OF FOOTINGS AND AS OTHERWISE NOTED, SPLICE TOP STEEL AT MIDSPAN AND BOTTOM STEEL AT SUPPORTS.
- EXCEPT WHERE SPECIFIED OTHERWISE, BEAR 4" AND 6" MASONRY PARTITION WALLS ON SLAB-ON-GRADE. BEAR ALL MASONRY WALLS ON FOOTINGS AS DETAILED.
- THE SPECIFICATOINS FOR CAST-IN-PLACE CONCRETE ARE BASED ON CSA A23.1. THE CONTRACTOR AND SUPPLIER ARE REQUIRED TO ADHERE TO CSA STANDARDS CAN-A23.1 AND A23.2 REGARDING CONCRETE MIX COMPONENTS, PLACING, CURING, AND TESTING.
- A CONCRETE READY-MIX SUPPLIER MUST BE A MEMBER IN GOOD STANDING OF THEIR PROVINCIAL READY-MIX CONCRETE ASSOCIATION.
- ALL STRUCTURAL CONCRETE MUST HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 25 MPa FOR UNEXPOSED ELEMENTS AND 30MPa FOR EXPOSED ELEMENTS, UNLESS OTHERWISE SPECIFIED.
- THE FOLLOWING CSA A23.1 EXPOSURE CLASSES ARE LISTED BELOW, AND THE CONCRETE SUPPLIER AND CONTRACTOR RESPONSIBLE FOR CONCRETE MUST SUPPLY CONCRETE THAT SATISFIES THESE REQUIREMENTS.
  - CLASS C1: CONCRETE EXPOSED TO CHLORIDES INCLUDING EXPOSURE TO DE-ICING SALTS DURING WINTER MONTHS. THIS INCLUDES BUT IS NOT LIMITED TO REINF. FOOTINGS, WALLS AND PIERS IN THE VICINITY OF PARKING LOTS OR PATHWAYS WHERE USE OF DEICING SALTS CAN BE REASONABLY EXPECTED OVER A 50 YR. SERVICE LIFE; IF C-1 APPLIES, COMBINE IT WITH ONE OF THE FOLLOWING EXPOSURE CLASSES AS PER SENTENCE 2C AS NECESSARY.
  - CLASS N: FOOTINGS WITH MIN. FROST COVER TO TOP OF FOOTINGS (SEE DETAILS/SECTIONS), INTERIOR SLABS, WALLS, PIERS, PEDESTALS AND COLUMNS (HEATED BUILDING ONLY, FOR UNHEATED BUILDINGS SEE NOTE "c").
  - CLASS F-2: EXTERIOR FOUNDATION WALLS, FOOTINGS WITH INADEQUATE FROST COVER, RETAINING WALLS , EXTERIOR PIERS/COLUMNS.
  - CLASS C-2 SIDEWALKS AND CURBS.

**ISSUED FOR PERMIT ONLY**

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CLIENT:  
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PROJECT NAME  
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PROJECT LOCATION  
**1041 Fern Avenue,  
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DRAWING TITLE:  
**GENERAL NOTES**

SCALE: <b>N.T.S</b>	DRAWN BY: <b>A.R</b>	CHECKED BY: <b>G.T</b>
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DRAWING #: <b>AD-100</b>	DATE: <b>APR. 2024</b>
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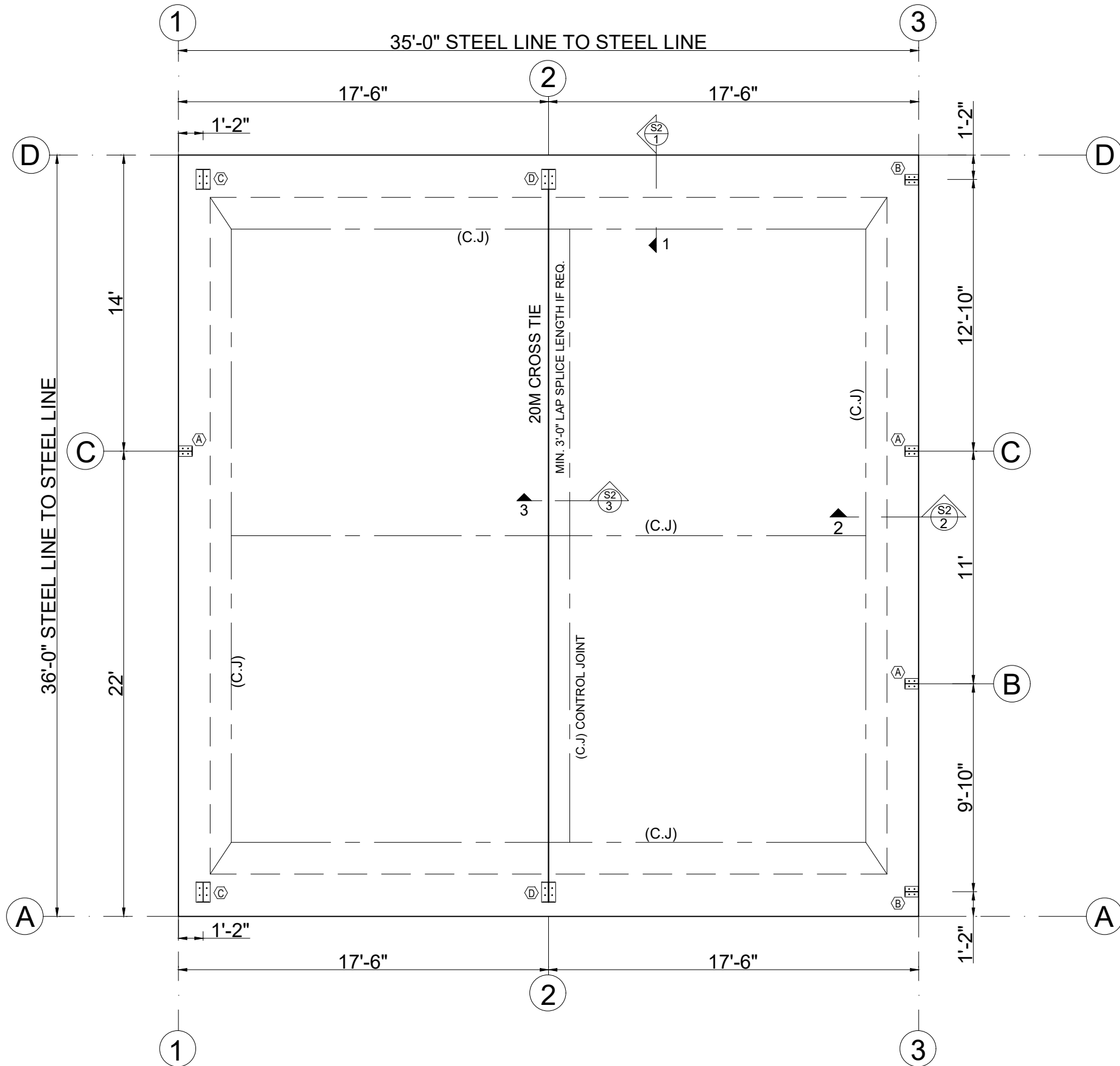
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- NOTE:
- 1- INSIDE THE BUILDING, 6" CONCRETE SLAB ON MIN 6" GRAN. 'B' BASE COMPACTED TO 98% S.P.M.D.D,  $f_c$  MIN. 25 MPa.
  - 2- IF THE SURROUNDING INSULATION IS OMITTED AND THE NATURAL SOIL BASE IS NOT FREE DRAINING GRANULAR MATERIAL, AND/OR THE WATER TABLE IS NOT BELOW FROST DEPTH, THE SLAB-ON-GRADE MAY HEAVE. THIS WILL RESULT IN ADDITIONAL SLAB CRACKING DUE TO THERMAL EFFECTS. THIS WILL NOT BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
  - 3- FOUNDATION ASSUMED TO BE INSTALLED ON FREE DRAINING GRANULAR SOIL AND WATER TABLE ASSUMED TO BE BELOW FROST DEPTH. ASSUMPTIONS TO BE CONFIRMED BY GEOTECHNICAL CONSULTANT PRIOR TO PROCEEDING WITH WORK.
  - 4- BUILDING ASSUMED TO BE HEATED.



**FOUNDATION PLAN**  
N.T.S

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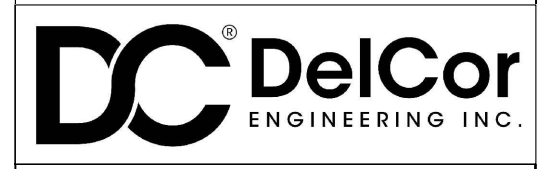
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**FOUNDATION PLAN**

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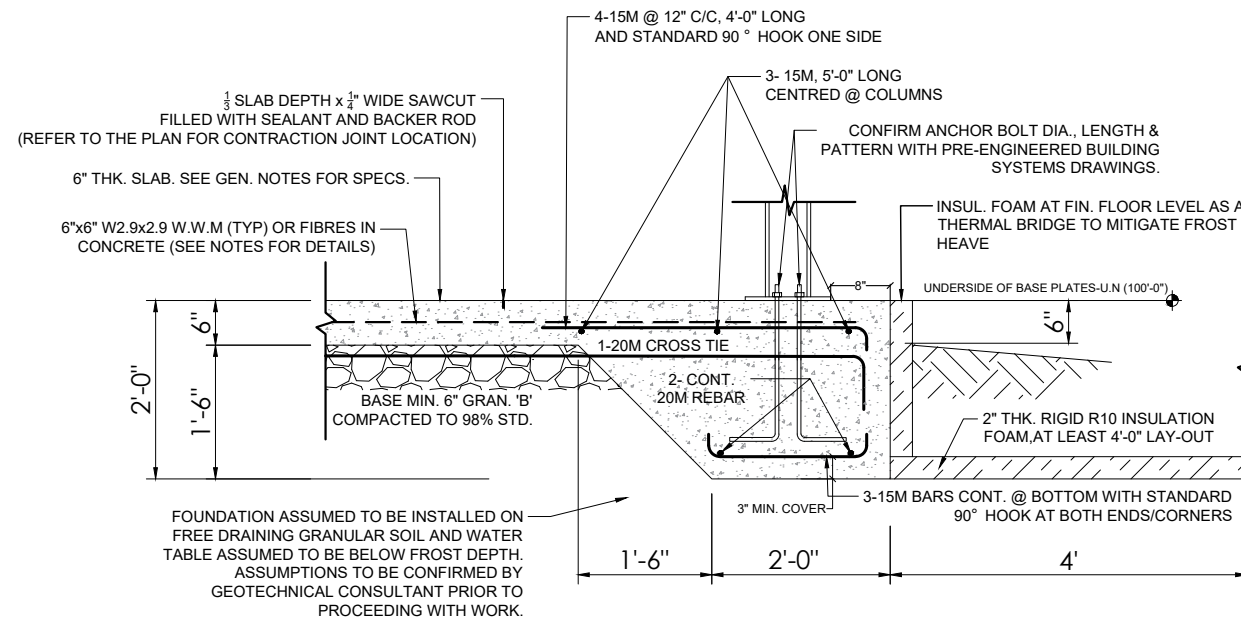
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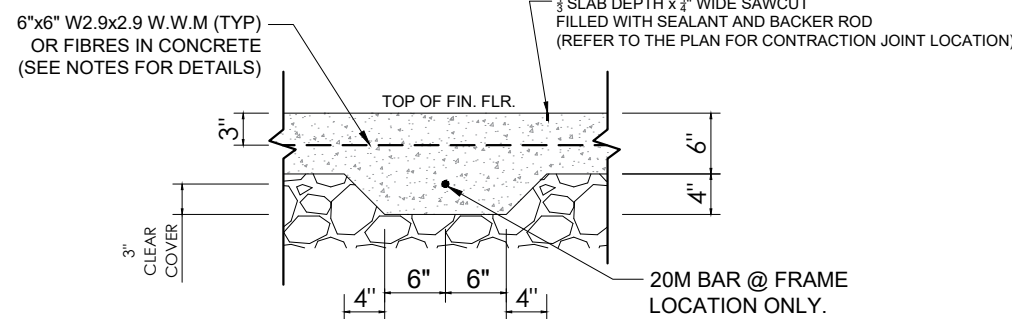


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**SECTION 1-1, LINES A-1-3 & D-1-3 (DETAIL ALONG SLAB PERIMETER)**

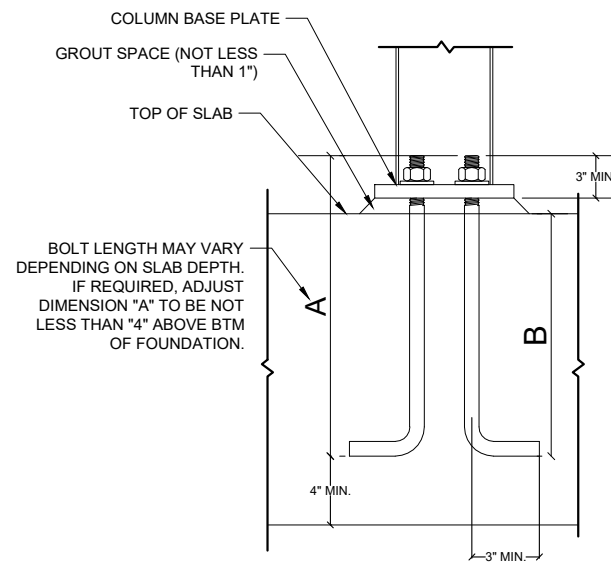
N.T.S



NOTE: BUILDING ASSUMED TO BE HEATED.

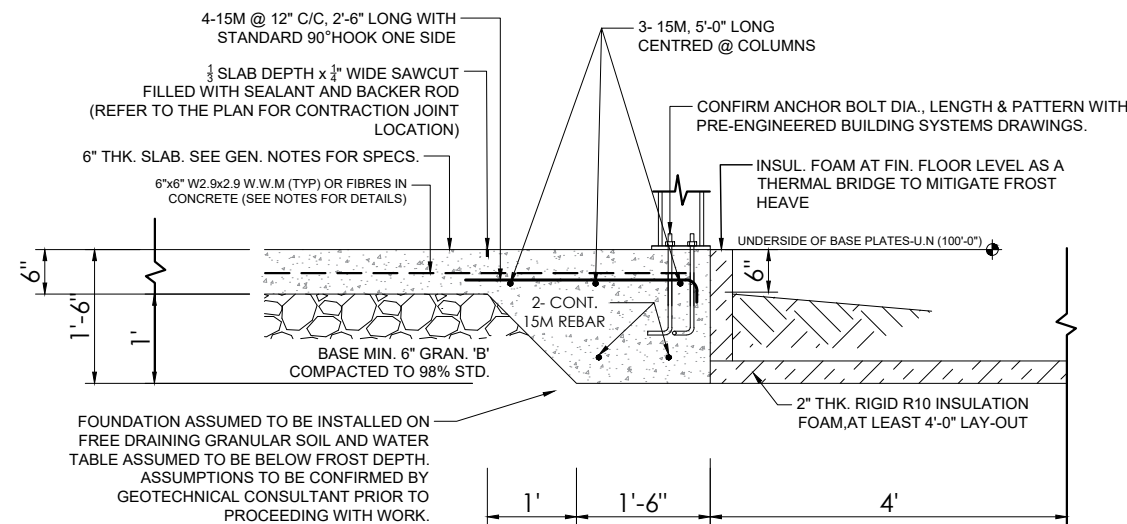
**SECTION 3-3, (TYPICAL SECTION OF TIE-ROD @ FRAME LOCATIONS)**

N.T.S



**a TYPICAL L-SHAPED ANCHOR BOLTS**

N.T.S



**SECTION 2-2, LINES 1-A-D & 3-A-D (DETAIL ALONG SLAB PERIMETER)**

N.T.S

**ANCHOR ROD SCHEDULE (See Base Plate Details by STEEL BUILDINGS COMPANY DRAWINGS)**

BASE PLATE	ANCHOR ROD SIZES & QUANTITY
A	(4) -3/4" Ø - A = 16" lg. (B=12" lg. EMBEDMENT) F1554 Gr. 36 STANDARD L-SHAPED
B	(4) -3/4" Ø - A = 16" lg. (B=12" lg. EMBEDMENT) F1554 Gr. 36 STANDARD L-SHAPED
C	(4) -3/4" Ø - A = 18" lg. (B=14" lg. EMBEDMENT) F1554 Gr. 36 STANDARD L-SHAPED
D	(4) -3/4" Ø - A = 18" lg. (B=14" lg. EMBEDMENT) F1554 Gr. 36 STANDARD L-SHAPED

REFER TO DRAWINGS BY STEEL BUILDINGS COMPANY FOR COLUMN BASE PLATE SIZES

**IMPORTANT NOTE:**

- IF THE SURROUNDING INSULATION IS OMITTED AND THE NATURAL SOIL BASE IS NOT FREE DRAINING GRANULAR MATERIAL, AND/OR THE WATER TABLE IS NOT BELOW FROST DEPTH, THE SLAB-ON-GRADE MAY HEAVE. THIS WILL RESULT IN ADDITIONAL SLAB CRACKING DUE TO THERMAL EFFECTS. THIS WILL NOT BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- MINIMUM 3" COVER AT THE BOT. OF FOUNDATION SHOULD BE PROVIDED.

**ISSUED FOR PERMIT ONLY**

Rev.	Date	By	Description

CLIENT:  
**Agostino Dimarsico**

PROJECT NAME:  
**Agostino Dimarsico**

PROJECT LOCATION:  
**1041 Fern Avenue,  
Innisfil, ON, L9S 4R7**

DRAWING TITLE:  
**FOUNDATION DETAILS**

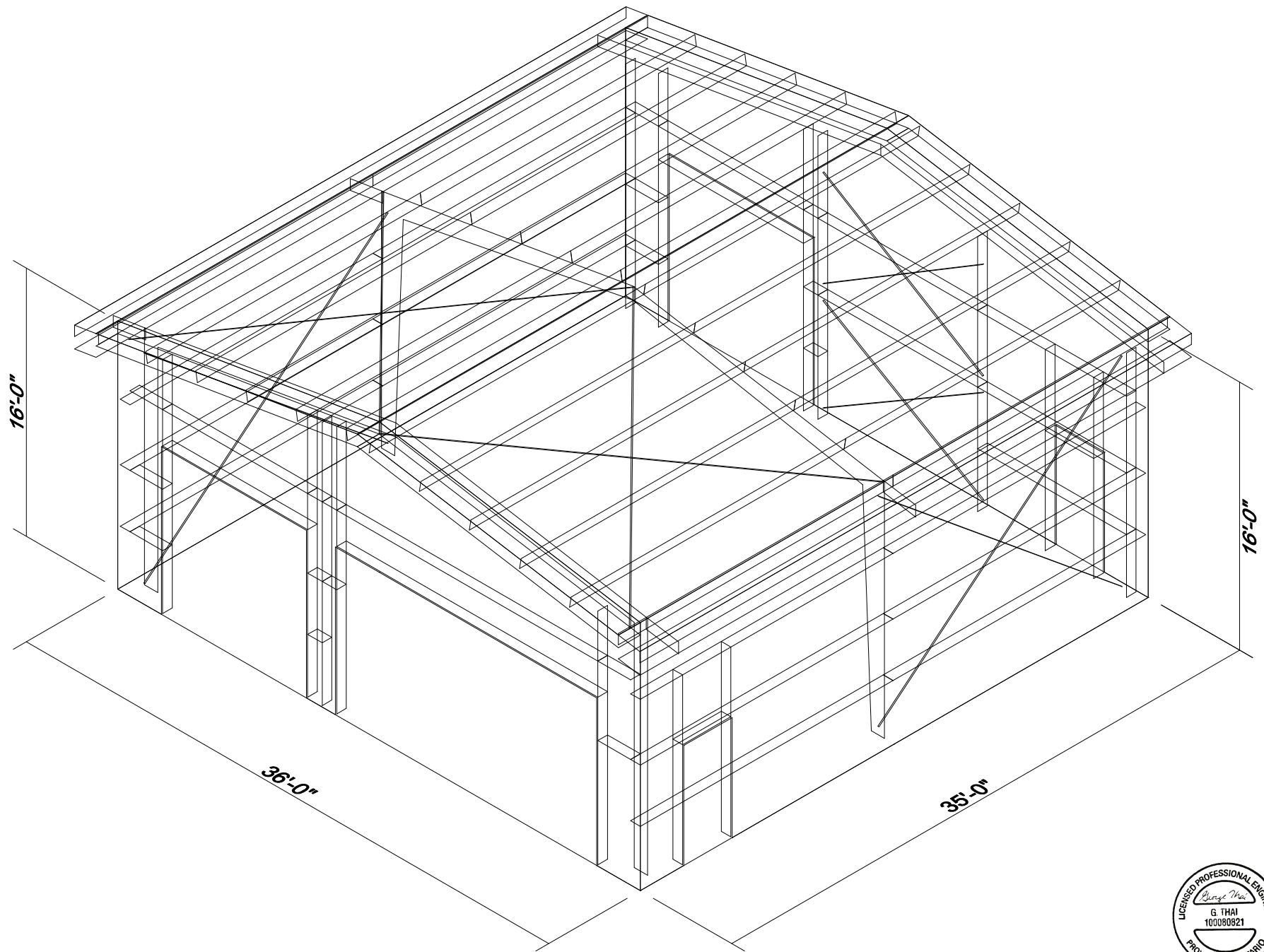
SCALE: **N.T.S**      DRAWN BY: **A.R**      CHECKED BY: **G.T**

DRAWING #: **AD-102**      DATE: **APR. 2024**

ENGINEER STAMP:  
  
**04-05-2024  
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**DC DelCor**  
ENGINEERING INC.

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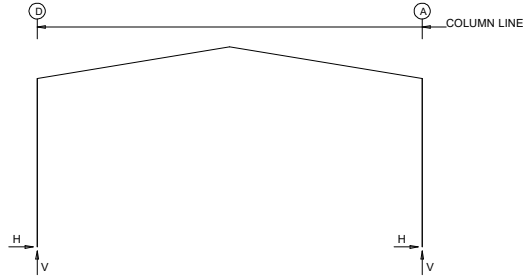


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FRAME LINES: 1 2



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	----Dead----		--Collateral--		----Live----		----Snow----		--Wind_Left1--		--Wind_Right1--	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	0.2	1.0	0.2	0.6	1.1	4.4	2.7	10.6	-1.7	-4.1	0.7	-2.5
1	A	-0.2	1.0	-0.2	0.6	-1.1	4.4	-2.7	10.6	-0.7	-2.5	1.7	-4.1
Frame Line	Column Line	--Wind_Left2--		--Wind_Right2--		--Wind_Long1--		--Wind_Long2--		--Seismic_Left--		--Seismic_Right--	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	-2.0	-2.1	0.4	-0.5	0.2	-4.4	0.0	-3.9	-0.7	-0.6	0.7	0.6
1	A	-0.4	-0.5	2.0	-2.1	0.0	-2.5	-0.2	-3.0	-0.7	0.6	0.7	-0.6
Frame Line	Column Line	--Seismic_Long		F1UNB_SL_L-		F1UNB_SL_R-							
		Horz	Vert	Horz	Vert	Horz	Vert						
1	D	0.0	-2.4	2.0	9.5	2.0	6.4						
1	A	0.0	0.0	-2.0	6.4	-2.0	9.5						
Frame Line	Column Line	----Dead----		--Collateral--		----Live----		----Snow----		--Wind_Left1--		--Wind_Right1--	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	0.4	1.6	0.4	1.3	2.6	8.7	6.3	20.8	-2.3	-6.2	0.8	-4.1
2	A	-0.4	1.6	-0.4	1.3	-2.6	8.7	-6.3	20.8	-0.8	-4.1	2.3	-6.2
Frame Line	Column Line	--Wind_Left2--		--Wind_Right2--		--Wind_Long1--		--Wind_Long2--		--Seismic_Left--		--Seismic_Right--	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	-2.7	-2.3	0.4	-0.2	0.4	-5.8	0.2	-5.2	-1.3	-1.1	1.3	1.1
2	A	-0.4	-0.2	2.7	-2.3	-0.2	-5.1	-0.4	-5.7	-1.2	1.1	1.2	-1.1
Frame Line	Column Line	--Seismic_Long		F2UNB_SL_L-		F2UNB_SL_R-							
		Horz	Vert	Horz	Vert	Horz	Vert						
2	D	0.0	-2.4	4.7	18.5	4.7	12.6						
2	A	0.0	-2.2	-4.7	12.6	-4.7	18.5						

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in)		Thick	AFF/BFF (in)
				Width	Length		
1	D	4	0.750	8.000	11.50	0.500	0.0
1	A	4	0.750	8.000	11.50	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in)		Thick	AFF/BFF (in)
				Width	Length		
2	D	4	0.750	8.000	11.50	0.500	0.0
2	A	4	0.750	8.000	11.50	0.500	0.0

TBD

36'-0" x 35'-0" x 16'-0"

DATE: 4/3/24

ENG: DWN: APPD:

REVISION: 0

F.O. Innisfil ON36x35x16

CITY: TBD ST / PW: TBD

REVISION HISTORY

REV	DESCRIPTION	DATE

DRAWING STATUS

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FOR CONSTRUCTION: FINAL DRAWINGS.

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ENDWALL COLUMN:		BASIC COLUMN REACTIONS (k)									
Frm Line	Col Line	Dead Vert	Wind Press Horiz	Wind Suct Horiz	Wind Seis Long Horiz	Wind Left1 Horiz	Wind Right1 Horiz	Wind Left2 Horiz	Wind Right2 Horiz	Seis Left Vert	Seis Right Vert
1	C	0.2	-1.8	1.2	0.1						
Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Horiz	Wind Right1 Horiz	Wind Left2 Horiz	Wind Right2 Horiz	Seis Left Vert	Seis Right Vert
3	A	0.4	0.2	1.4	3.4	0.0	-0.8	0.0	-1.2	0.0	-0.1
3	B	0.5	0.3	2.4	5.7	-0.9	-3.5	0.0	0.3	-0.9	-2.4
3	C	0.6	0.4	3.1	7.4	0.0	-0.3	0.9	-3.8	0.0	1.0
3	D	0.4	0.2	1.7	4.0	0.0	-0.8	0.0	-1.6	0.0	0.0
Frm Line	Col Line	Wind Press Horiz	Wind Suct Horiz	Wind Long1 Horiz	Wind Long2 Horiz	Seis Left Horiz	Seis Right Horiz	Seis Long Horiz	Seis Long Vert		
3	A	-2.1	-1.3	0.4	1.3	0.0	-1.0	0.0	0.0	-2.8	
3	B	-1.0	0.0	0.7	0.0	0.0	-1.7	-0.2	-1.4	-1.3	
3	C	-1.2	0.0	0.8	0.0	0.2	-1.9	0.0	-2.0	0.0	
3	D	-0.6	0.0	0.5	0.0	0.0	-0.8	0.0	-1.3	0.0	
Frm Line	Col Line	E2UNB_SL_L- Horiz	E2UNB_SL_R- Horiz	E2UNB_SL_L- Vert	E2UNB_SL_R- Vert						
3	A	0.0	3.4	0.0	1.7						
3	B	0.0	6.0	0.0	2.6						
3	C	0.0	4.1	0.0	7.0						
3	D	0.0	2.0	0.0	4.1						

ENDWALL COLUMN:		ANCHOR BOLTS & BASE PLATES					
Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate Width	Base Plate Length	Thick	AFF/BFF (in)
1	C	4	0.750	6.000	7.875	0.375	0.0
3	A	4	0.750	6.000	7.875	0.375	0.0
3	B	4	0.750	6.000	7.875	0.375	0.0
3	C	4	0.750	6.000	7.875	0.375	0.0
3	D	4	0.750	6.000	7.875	0.375	0.0

ANCHOR BOLT SUMMARY			
Qty	Locate	Dia (in)	Type
20	Endwall	3/4"	
16	Frame	3/4"	


BUILDING BRACING REACTIONS										
Loc	Wall Line	Col Line	Reactions in plane of wall				Panel Shear (lb/ft)	Wind	Seis	Note
			Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert				
L_EW	1									(h)
F_SW	A	2.3	1.6	*	2.8	*				
R_EW	3	B,C	Bracing, see EW reactions							
B_SW	D	2.1	1.6	*	2.8	*				

(h) Rigid frame at endwall

\*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

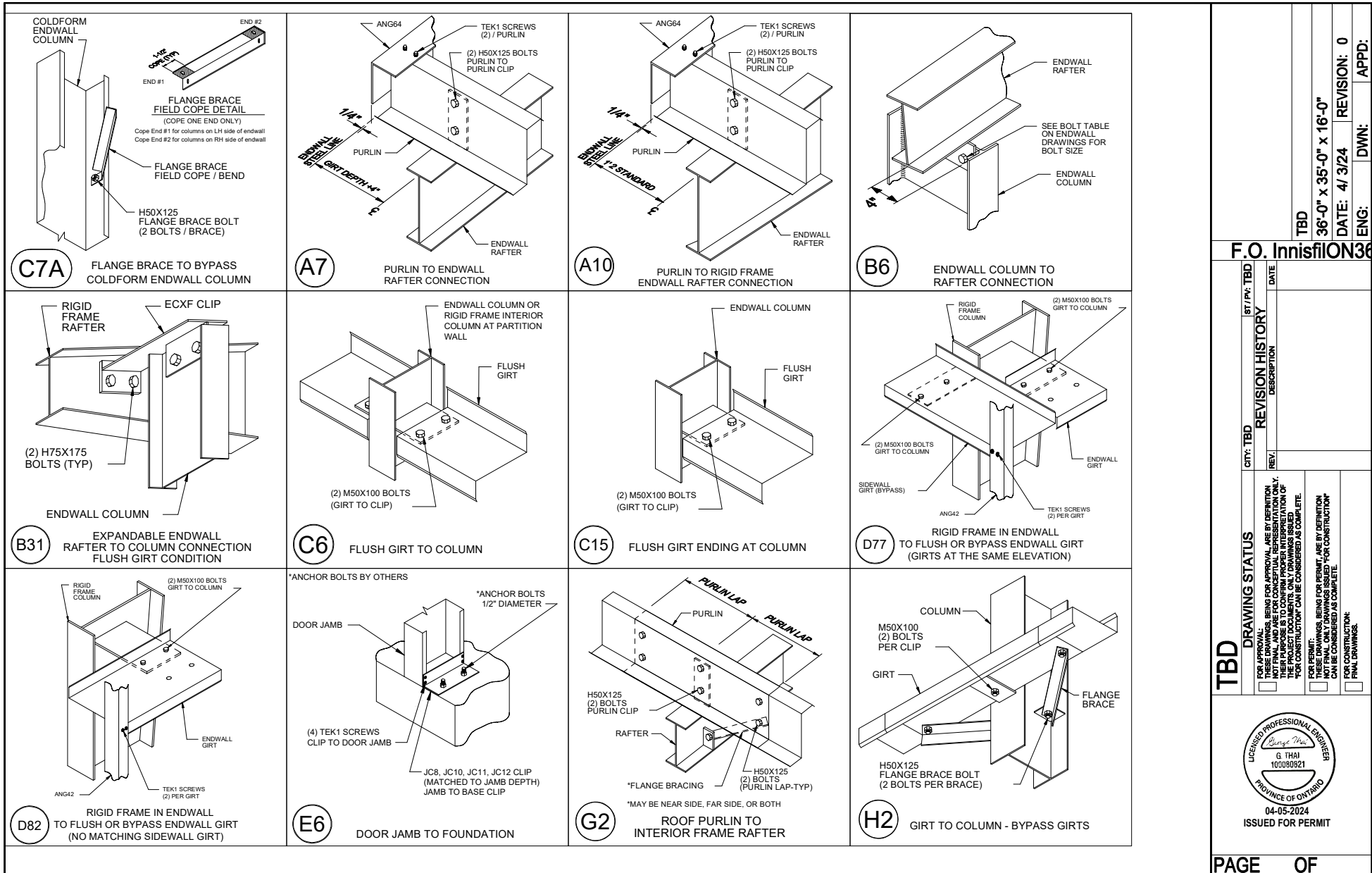
Reactions for seismic represent shear force, V  
Reaction values shown are unfactored

+DESIGN INFORMATION		
1. All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.		
2. Positive reactions are shown in the sketch. Foundation loads are in opposite directions.		
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.		
4. Building reactions are based on the following building data:		
DESIGN CRITERIA	SEISMIC CRITERIA	DEFLECTION LIMITS
Width (ft) = 36	Seismic Importance = 1.00	ENDWALL COLUMN L / 180
Length (ft) = 35	Risk Category = II - Normal	ENDWALL RAFTER (Live) L / 180
Eave Height (ft) = 16		ENDWALL RAFTER (Wind) L / 180
Roof Slope (rise/12) = 2.0:12	Mapped Spectral Response Accelerations	WALL GIRTS L / 90
Building Code = NBC 20	Sa (0.2,X) = 0.2590	PURLIN (LIVE) L / 180
Local Code (State/Prov) = NBC 20	Sa (0.5,X) = 0.2700	PURLIN (WIND) L / 180
Dead Load (psf) = 2.25	Sa (1.0,X) = 0.1650	WALL PANEL L / 90
Collateral Load (psf) = 3.00	Sa (2.0,X) = 0.0799	ROOF PANEL (Live) L / 180
Roof Live Load (psf) = 21.00	Sa (5.0,X) = 0.0213	ROOF PANEL (Wind) L / 120
Frame Live Load (psf) = 21.00	Sa (10.0,X) = 0.0067	Main Frame (Horiz) H / 60
		Main Frame (Vert) L / 180
Snow:	Site Class = D	WIND BRACING H / 60
Ground Snow Load (psf) = 52.25		Main Frame (Crane) H / 100
Snow Importance = 1.0000	----Base Shear----	Main Frame (Seismic) H / 40
Associated Rain Load (psf) = 8.36	Expanded Formula = S(Ta)*Mv*le*W/(Rd*Ro)	SEISMIC BRACING H / 40
Wind Exposure Factor = 1.00	Longitudinal Base Shear (k) = 5.68	PARTITION COLUMN L / 120
Slippery Roof = N	Transverse Base Shear (k) = 5.25	PARTITION GIRT L / 120
Roof Snow Load (psf) = 50.16		PARTITION PANEL L / 120
Wind:	--Seismic Response Coefficients--	
Wind (1/50) (psf) = 7.52	Frame = 0.139	
Risk Category = II - Normal	FSW = 0.139	
Importance - Wind = 1.00	BSW = 0.139	
Wind Exposure = O		
Enclosure Classification = 2	--Response Modification Factors--	
--Internal Pressure Coefficients--	Frame = 1.5	
Pressure = 0.30	FSW = 1.5	
Suction = -0.45	BSW = 1.5	
---Components & Cladding---		
Design Pressure:		
Pressure (psf) = 18.33		
Suction (psf) = -18.33		
Equivalent Static Force Procedure.		

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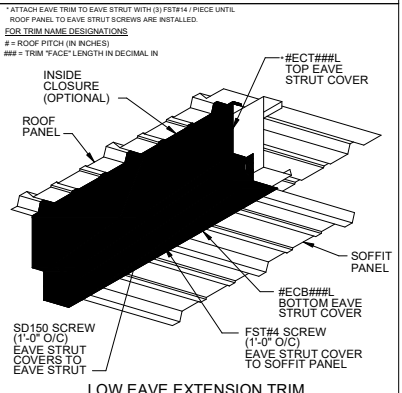
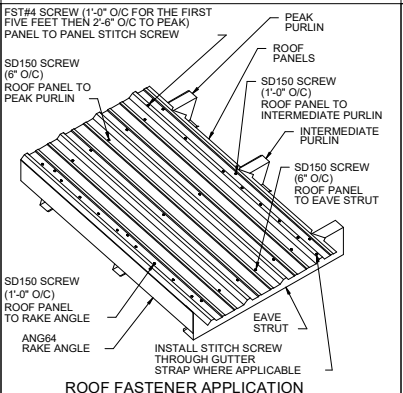
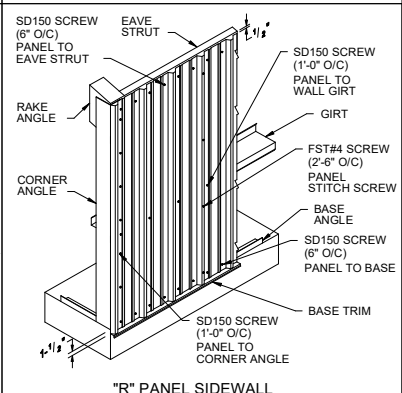
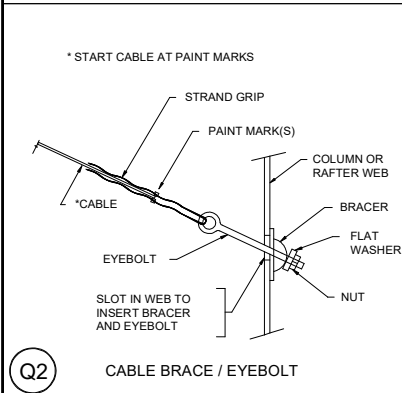
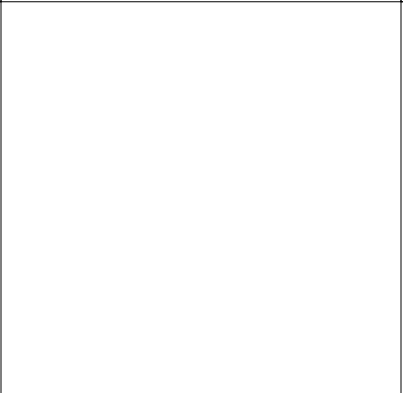
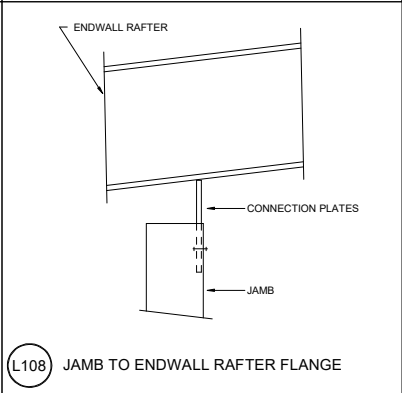
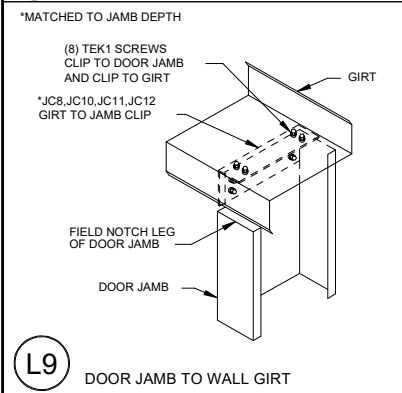
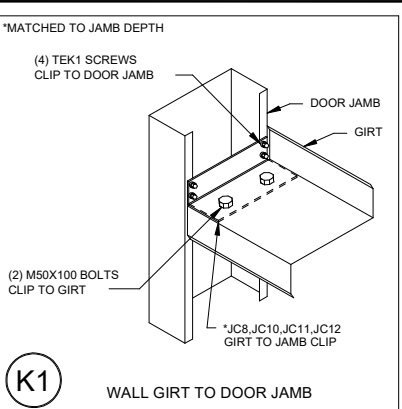
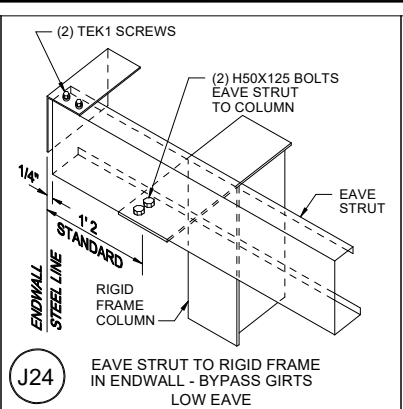
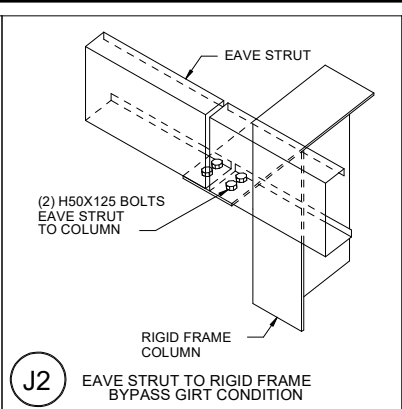
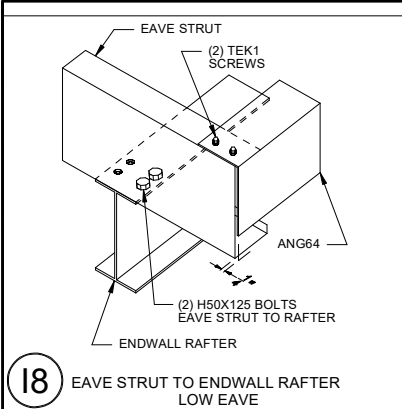
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LICENSED PROFESSIONAL ENGINEER  
 G. THAI  
 10090921  
 PROVINCE OF ONTARIO  
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DATE	DESCRIPTION

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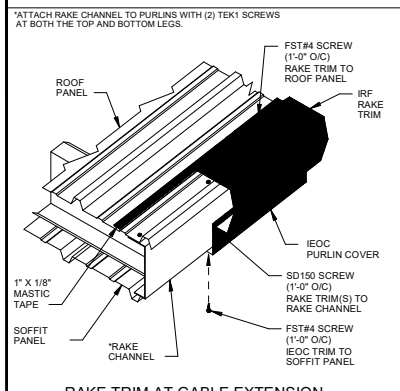
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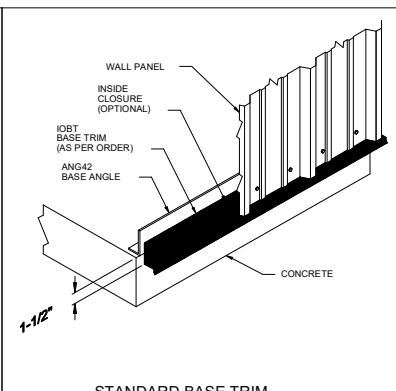
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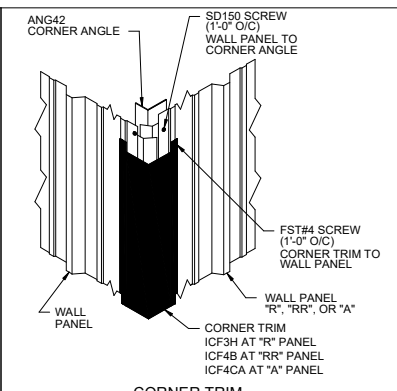




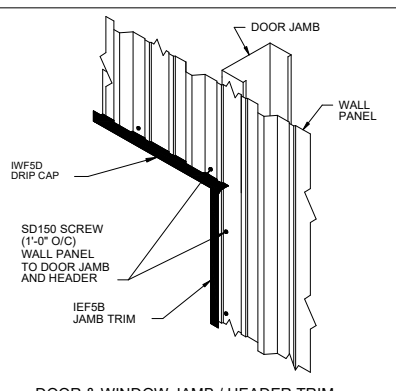
RAKE TRIM AT GABLE EXTENSION



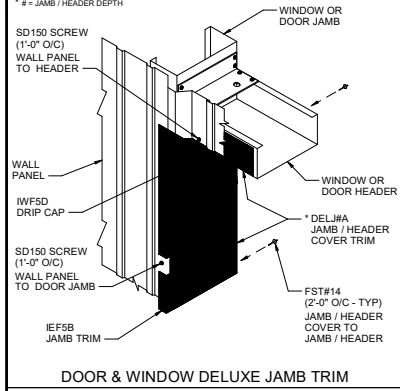
STANDARD BASE TRIM



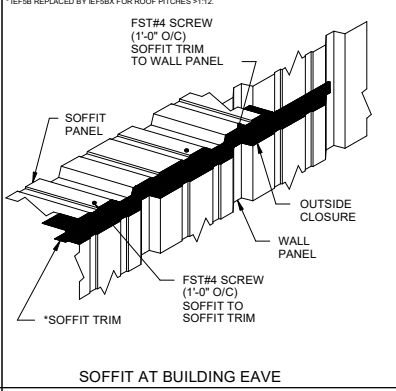
CORNER TRIM



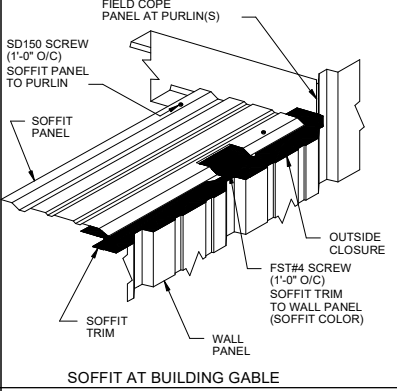
DOOR & WINDOW JAMB / HEADER TRIM



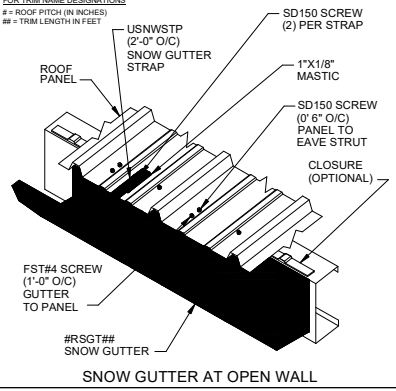
DOOR & WINDOW DELUXE JAMB TRIM



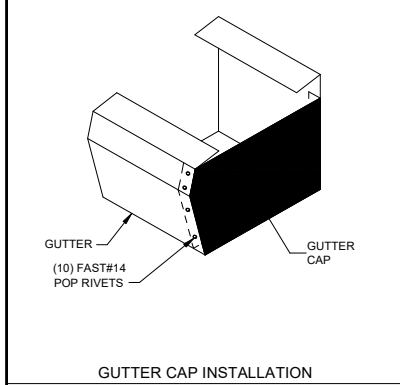
SOFFIT AT BUILDING EAVE



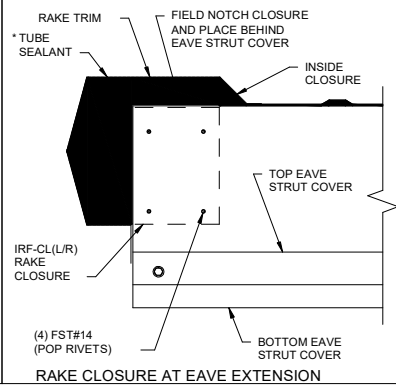
SOFFIT AT BUILDING GABLE



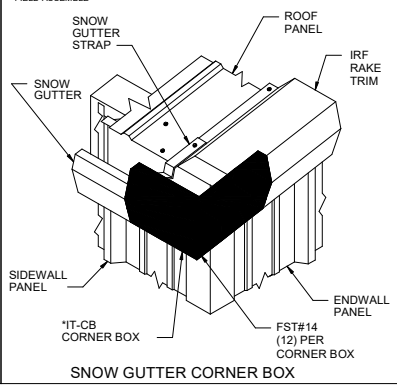
SNOW GUTTER AT OPEN WALL



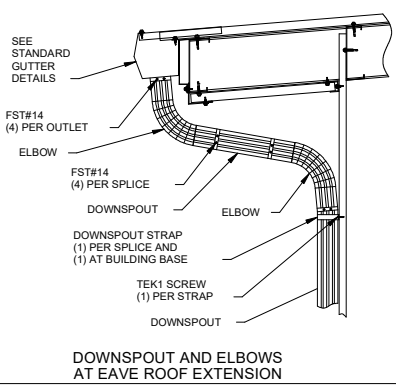
GUTTER CAP INSTALLATION



RAKE CLOSURE AT EAVE EXTENSION



SNOW GUTTER CORNER BOX



DOWNSPOUT AND ELBOWS AT EAVE ROOF EXTENSION

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<b>DRAWING STATUS</b>	<b>DRAWING HISTORY</b>
<p>FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION PRELIMINARY AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION PRELIMINARY AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR CONSTRUCTION: THESE DRAWINGS, BEING FOR CONSTRUCTION, ARE BY DEFINITION FINAL AND ARE FOR CONSTRUCTION. THE PROJECT DOCUMENTS ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.</p>	<p>CITY: TBD</p> <p>ST/PK: TBD</p>

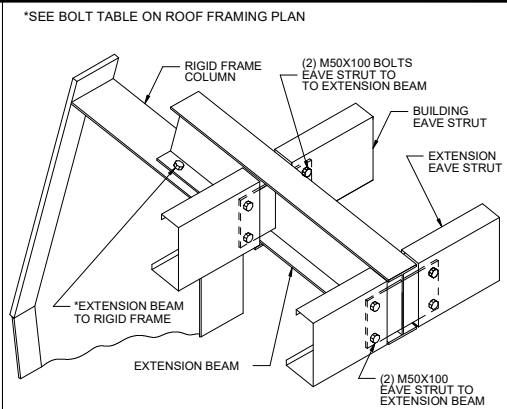
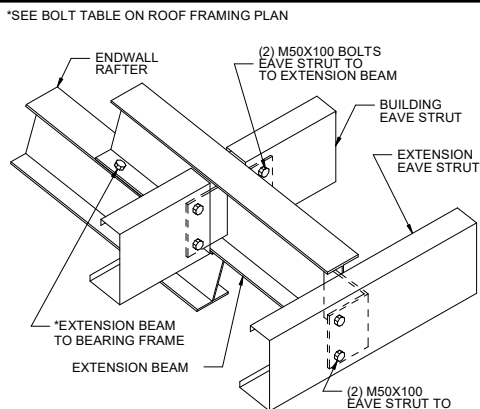
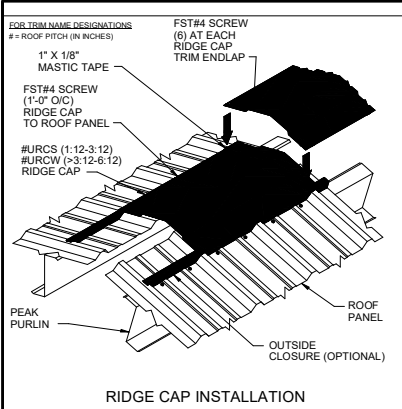
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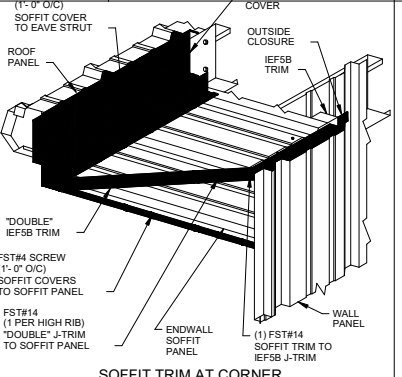
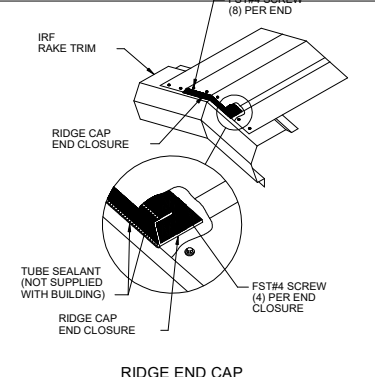
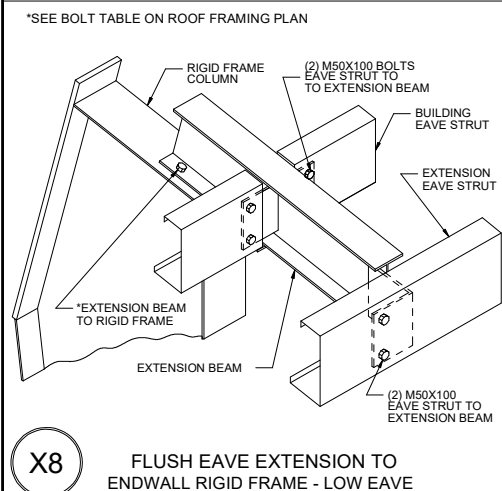
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ISSUED FOR PERMIT



**X6** FLUSH EAVE EXTENSION TO BEARING FRAME ENDWALL AT LOW EAVE

**X7** FLUSH EAVE EXTENSION TO RIGID FRAME - LOW EAVE



**X8** FLUSH EAVE EXTENSION TO ENDWALL RIGID FRAME - LOW EAVE

TBD	36'-0" x 35'-0" x 16'-0"	REVISION: 0	APPD:
ENG:	DATE: 4/3/24	DWN:	

F.O. Innisfil ON36x35x16

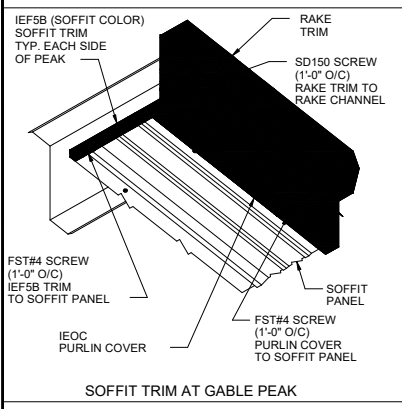
REV	DATE	DESCRIPTION

<b>DRAWING STATUS</b>	<b>DRAWING HISTORY</b>
<p>FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p>	<p>CITY: TBD</p> <p>ST / PW: TBD</p>

**TBD**

ISSUED FOR PERMIT



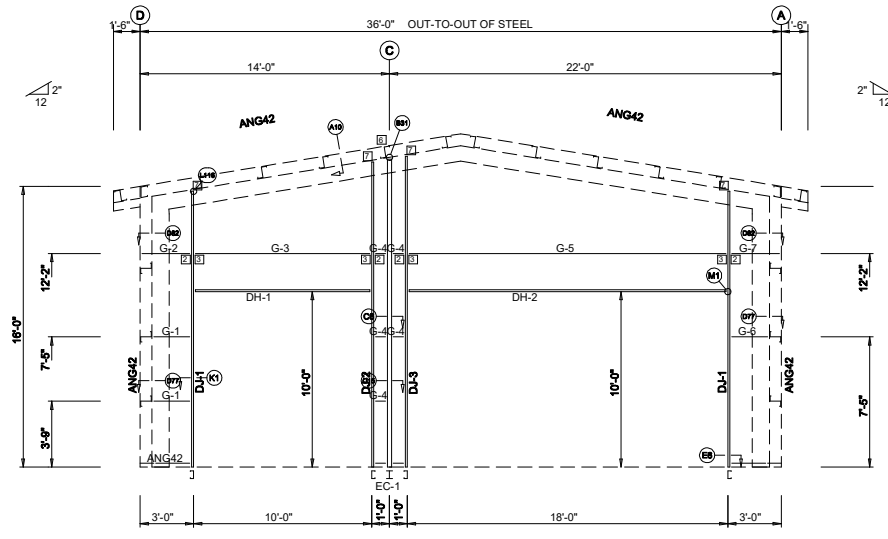


<b>TBD</b> DRAWING STATUS <input type="checkbox"/> FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE. <input type="checkbox"/> FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE. <input type="checkbox"/> FOR CONSTRUCTION: FINAL DRAWINGS.		CITY: TBD REV:	ST / PW: TBD REVISION HISTORY DESCRIPTION   DATE
		DATE: 4/3/24 ENG:	REVISION: 0 APPD:
36'-0" x 35'-0" x 16'-0" TBO		F.O. Innisfil ON36x35x16	

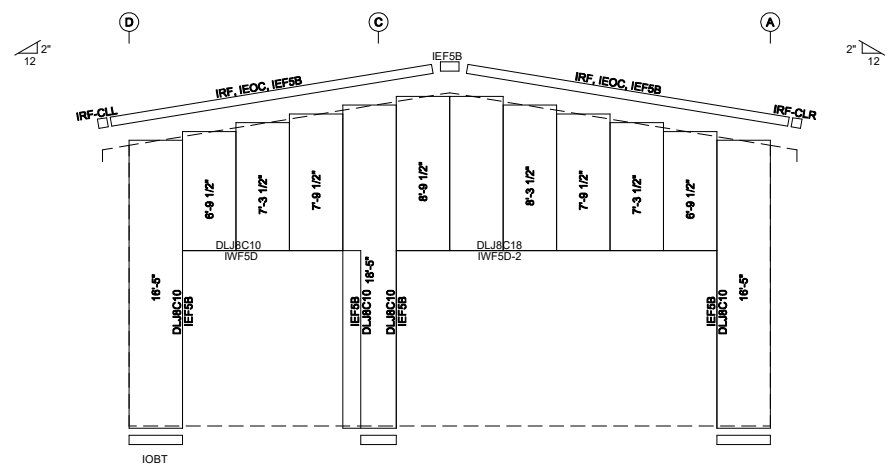


GENERAL	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD
All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 2.1)	Hot-Rolled Mill Sections	A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class 1 A 792, Gr. 80	Fy = 50 ksi Fy = 80 ksi
<b>DESIGN RESPONSIBILITY</b> The manufacturer is responsible only for the structural design of the Metal Building System it sells to the purchaser / customer. Neither the manufacturer nor the manufacturer's engineer is the design professional or engineer of record for the construction project. The manufacturer is not responsible for the design of any component or materials not sold by it, or their interface and connection with Metal Building System unless such design responsibility is specifically required by the order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.1)	Structural Steel Plates	A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi
	Structural Steel Bars	A 572 or A 529	Fy = 55 ksi	High Strength Bolts	F3125: A 325-N A 490-N	Fy = 92 or 81 ksi N/A
	Cold Formed Light Gauge Shapes	A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (if supplied)	A 36	Fy = 36 ksi
<b>FOUNDATION DESIGN AND ANCHOR BOLTS</b> The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.2). It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.2) U.S. - Anchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges). Canada - Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, December 2015, Clause 7.8.1	Cable Bracing	A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi
	Rod Bracing	A 36	Fy = 36 ksi			
	<b>CORRECTION OF ERRORS AND REPAIRS</b>					
	The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.14; CISC Code of Standard Practice, December 2015, Clause 7.15; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.10).					
<b>ADJACENT EXISTING BUILDINGS</b> The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.5)	<b>DRAWING DISCREPANCIES</b> In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 3.3; CISC Code of Standard Practice, December 2015, Clause 3.4; MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.1).					
<b>SHOP-PRIMED STEEL</b> All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud or other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, December 2015, Clauses 6.8 & 6.9; (MBMA 2018 Metal Building Systems Manual, Part IV, Section 4.2.4).	<b>DELIVERIES</b> Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am - 12pm (morning) and 12pm - 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.					
<b>ERECTION-GENERAL</b> The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, December 2015, Clause 7.3; (MBMA 2018 Metal Building System Manual, Part IV, Section 6.9). The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (2019), and / or the Seam-Lok Technical - Erection manual (2019) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field-erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection. The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.10.3; CISC Code of Standard Practices, December 2015, Clause 1.5; MBMA 2018 Metal Buildings System Manual, Part IV, Section 6.2.1.5).	<b>SHORTAGES</b> The purchaser / customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery. <b>1 to 3 loads...2 weeks 4 loads and over...3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.</b>					
	<b>FABRICATION ERRORS</b> The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.					
	<b>INVOICE PAYMENT</b> By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.					
<b>ERECTION TOLERANCES</b> U.S. : Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1/500. (AISC Code of Standard Practice for Steel Buildings and Bridges June 15, 2016 Section 7.13.1; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8) Canada: Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1/500. (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 29.3; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8)	<b>SAFETY PROCEDURES</b> The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).					
<b>BOLT TIGHTENING</b> The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; June 11, 2020). All high strength (ASTM F3125, A325, A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are compatible prior to the start of erection (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 23.7.2), (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9).	The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.					

<b>TBD</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>	<b>REV</b>	<b>DATE</b>	<b>DESCRIPTION</b>
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ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1  
PANELS: 26 Ga. R - TBD

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIAMETER	LENGTH
Columns/Raf	10	A325	3/4"	1 3/4"
Jamb	8	A325	1/2"	1 1/4"

CONNECTION PLATES		
FRAME LINE 1		
ID	QUAN	MARK/PART
2	4	CFFG5.5
3	4	CFFG2
6	1	ECXF
7	4	EJX0805

- GENERAL NOTES:**
1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
  2. See detail C7A for field coping of coldform endwall column flange braces.
  3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

DRAWING IS NOT TO SCALE	
TRIM COLORS	
EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.	

CITY: TBD	ST / PW: TBD
	REVISION HISTORY
REV:	DATE
DESCRIPTION	
TBD 36'-0" x 35'-0" x 16'-0" DATE: 4/ 3/24 REVISION: 0 ENG: DWN: APPD:	

CITY: TBD	ST / PW: TBD
	REVISION HISTORY
REV:	DATE
DESCRIPTION	
TBD 36'-0" x 35'-0" x 16'-0" DATE: 4/ 3/24 REVISION: 0 ENG: DWN: APPD:	

**TBD**

**DRAWING STATUS**

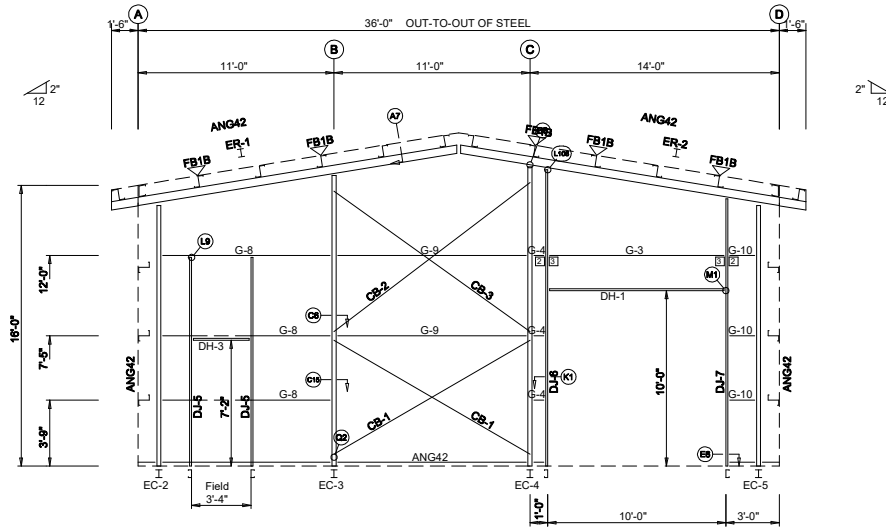
FOR APPROVAL: BEING FOR APPROVAL, ARE BY DEFINITION THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.

FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.

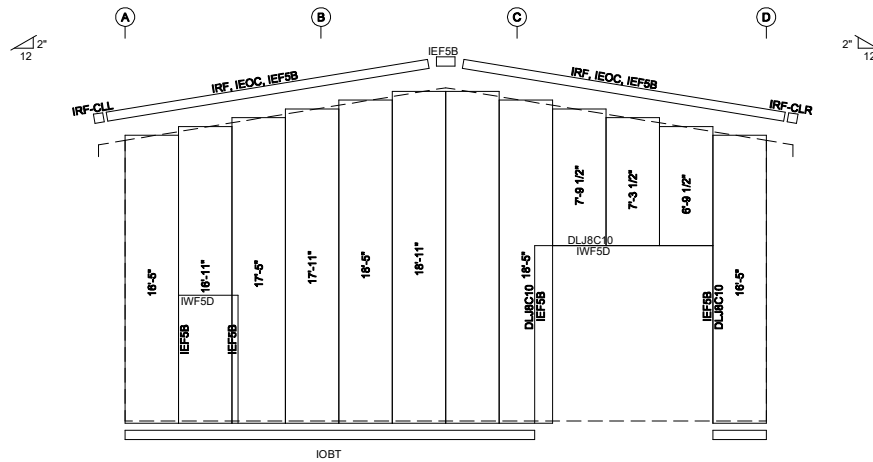
FOR CONSTRUCTION: FINAL DRAWINGS.

**LICENSED PROFESSIONAL ENGINEER**  
*George Miao*  
 G THAI  
 100090821  
 PROVINCE OF ONTARIO  
 04-05-2024  
 ISSUED FOR PERMIT

**PAGE OF**



ENDWALL FRAMING: FRAME LINE 3



ENDWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 26 Ga. R - TBD

**GENERAL NOTES:**

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

BOLT TABLE				
FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	1/2"	1 1/2"
Columns/Raf	2	A325	3/4"	1 3/4"
Jamb	2	A325	1/2"	1 1/4"

CONNECTION PLATES			
FRAME LINE 3			
ID	QUAN	MARK/PART	
2	2	CFFG5.5	
3	2	CFFG2	

FLANGE BRACE TABLE			
FRAME LINE 3			
ID	MARK	LENGTH	
1	FB1B	1'-2 3/8"	

DRAWING IS NOT TO SCALE

TRIM COLORS	
EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.	

TBD	ST / PW: TBD
CITY: TBD	REVISION HISTORY
REV:	DESCRIPTION
DATE	
TBD	
36'-0" x 35'-0" x 16'-0"	
DATE: 4/3/24	REVISION: 0
ENG:	DWN:
	APPD:

F.O. Innisfil ON36x35x16

TBD

**DRAWING STATUS**

- FOR APPROVAL: BEING FOR APPROVAL, ARE BY DEFINITION THESE DRAWINGS, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.
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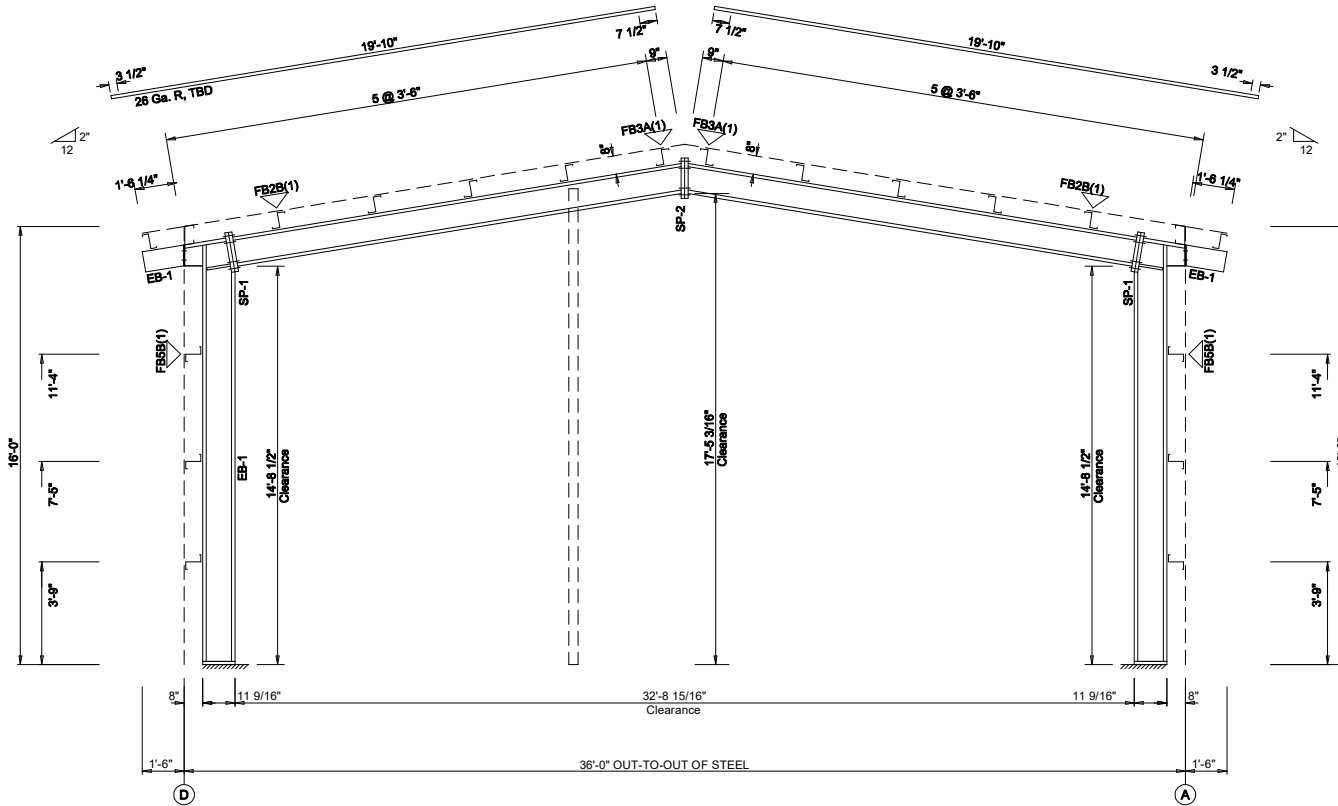
04-05-2024  
ISSUED FOR PERMIT

PAGE OF



SPLICE BOLT TABLE						
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.750	2.25
SP-2	4	4	0	A325	0.750	2.00

▽ FLANGE BRACES: Both Sides(U.N.)  
 FBxB(1)  
 B - L20X1/4  
 A - L15X1/8



BUILDING CROSS SECTION: FRAME LINE 1

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE

CITY: TBD REV:		ST / PW: TBD DATE	
<b>TBD</b>		<b>F.O. Innisfil</b>	
<b>DRAWING STATUS</b>		<b>REVISION HISTORY</b>	
<input type="checkbox"/> FOR APPROVAL: BEING FOR APPROVAL. ARE BY DEFINITION THESE DRAWINGS. NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. NOT FOR CONSTRUCTION. THE PROJECT DOCUMENTS ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.		DESCRIPTION	
<input type="checkbox"/> FOR PERMIT: THESE DRAWINGS BEING FOR PERMIT. ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.		36'-0" x 35'-0" x 16'-0"	
<input type="checkbox"/> FOR CONSTRUCTION: FINAL DRAWINGS.		DATE: 4/ 3/24	
		REVISION: 0	
		ENG:	
		DWN:	
		APPD:	

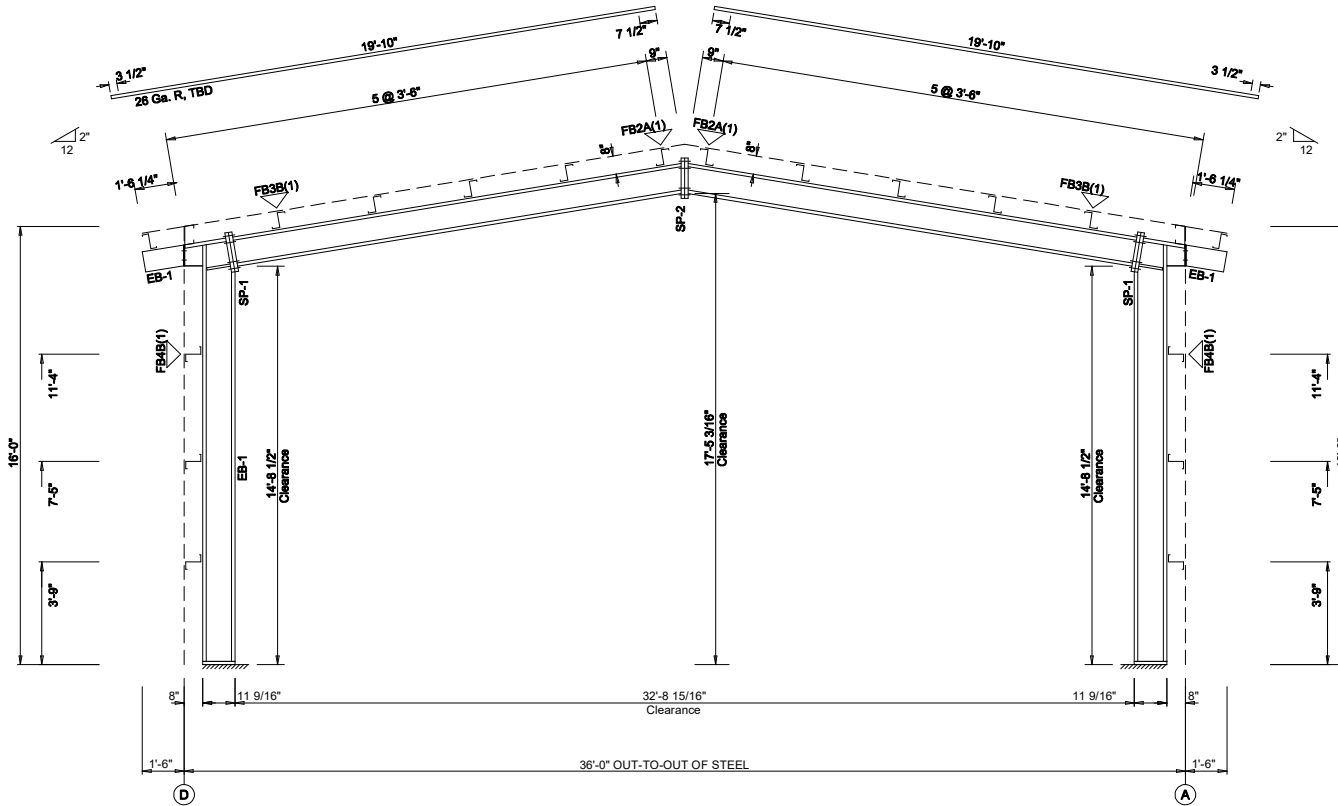


04-05-2024  
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SPLICE BOLT TABLE						
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.750	2.25
SP-2	4	4	0	A325	0.750	2.00

▽ FLANGE BRACES: Both Sides(U.N.)  
 FBxB(1)  
 B - L20X1/4  
 A - L15X1/8



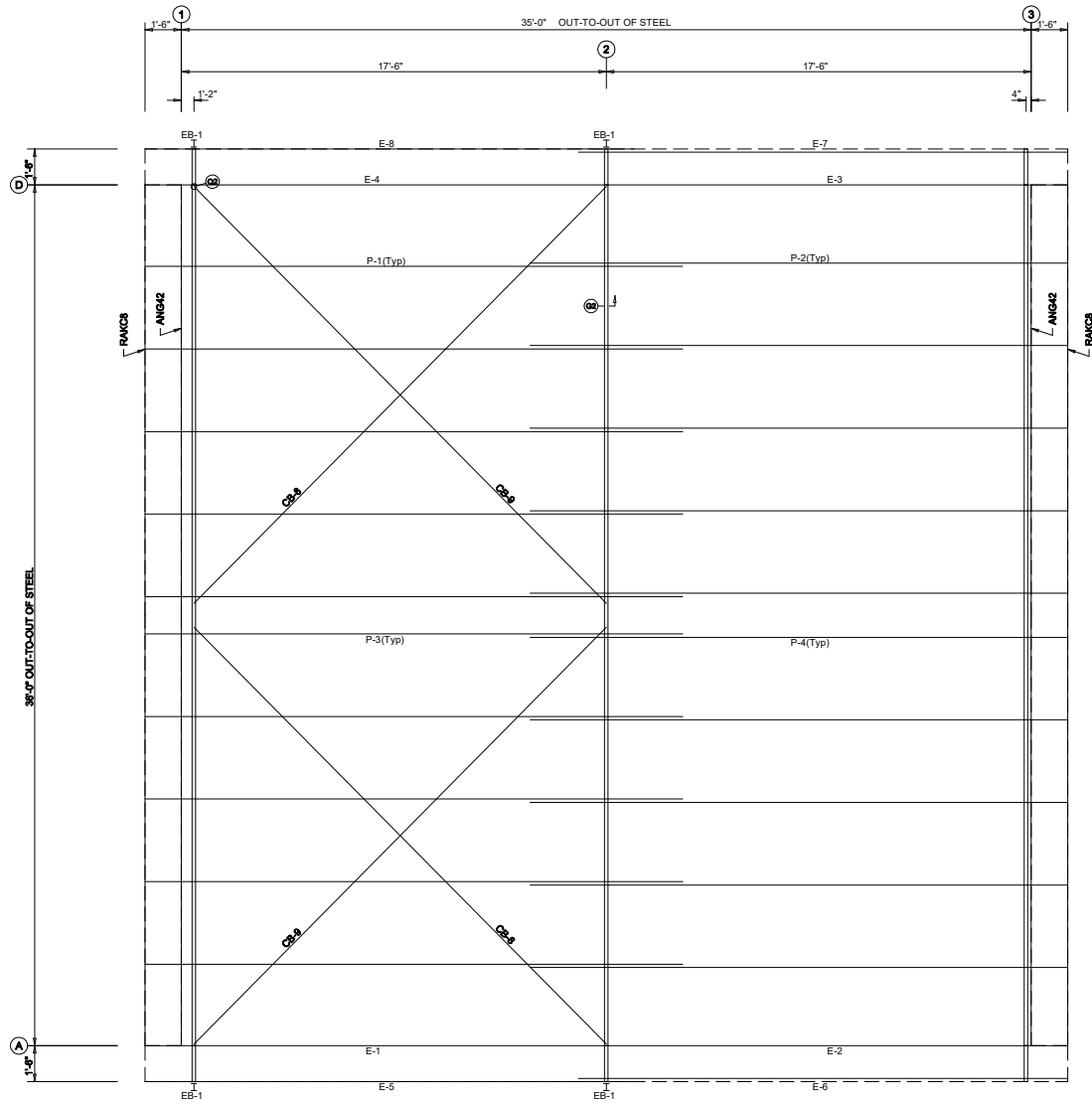
BUILDING CROSS SECTION: FRAME LINE 3

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE

CITY: TBD		ST / PW: TBD	DATE
REVISION HISTORY		DESCRIPTION	
<p><b>DRAWING STATUS</b></p> <p>FOR APPROVAL: BEING FOR APPROVAL, ARE BY DEFINITION THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR CONSTRUCTION: THESE DRAWINGS, BEING FOR CONSTRUCTION, ARE BY DEFINITION NOT FINAL, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p>			
<p><b>TBD</b></p> <p>LICENSED PROFESSIONAL ENGINEER  <i>G. THAI</i>          100080821          PROVINCE OF ONTARIO</p> <p>04-05-2024          ISSUED FOR PERMIT</p>		<p>36'-0" x 35'-0" x 16'-0"</p> <p>DATE: 4/2/24</p> <p>REVISION: 0</p> <p>APPD:</p>	
PAGE		OF	



EXTENSION/CANOPY BOLTS  
ROOF PLAN

MARK	QUAN	TYPE	DIA	LENGTH
EB-1	4	A325	1/2"	1 1/4"

**GENERAL NOTES:**

**PURLIN LAP**

- Screw Down Roof: Use TEK5WW screws in place of SD150 panel screws at all 10 gage purlins, eave struts, or roof joists.
- Standing Seam Roof: Use FST#6 in place of FST#1 clip to purlin screws at all 10 gage purlins, eave struts, or at roof joists.

**ROOF FRAMING PLAN**

DRAWING IS NOT TO SCALE

TBD	36'-0" x 35'-0" x 16'-0"	REVISION: 0	APPD:
DATE: 4/3/24	ENG:	DWN:	

F.O. Innisfil ON36x35x16

CITY: TBD	ST / PW: TBD	REVISION HISTORY	
		DESCRIPTION	DATE
REV:			

**DRAWING STATUS**

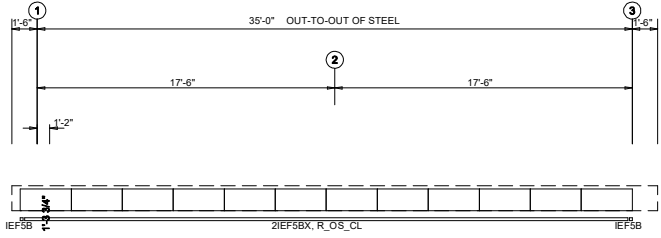
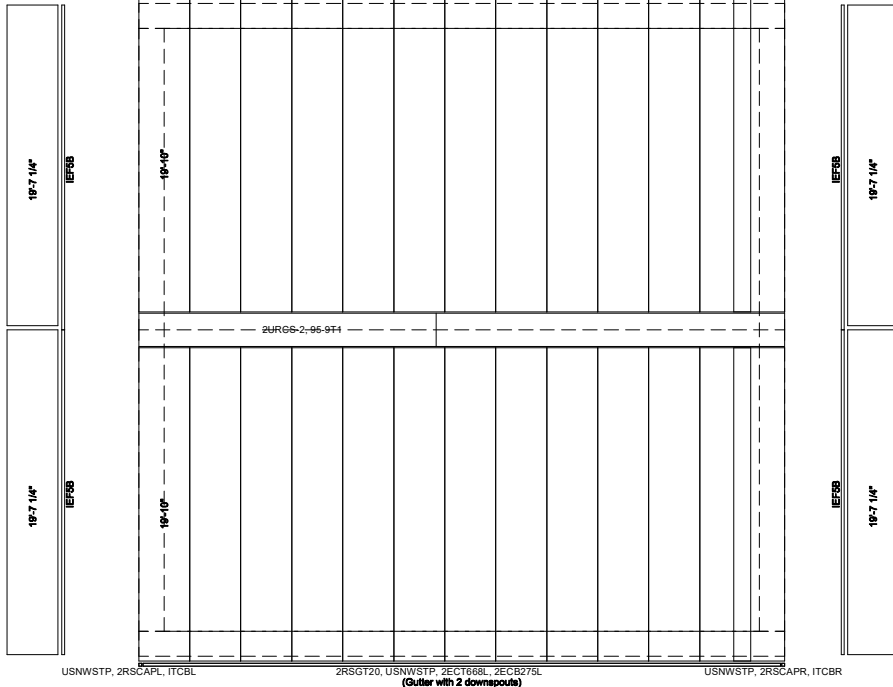
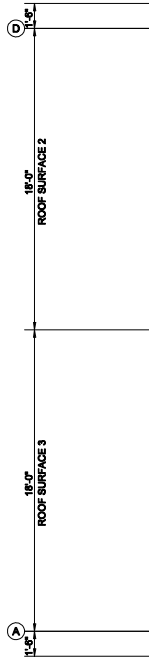
FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

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FOR CONSTRUCTION: FINAL DRAWINGS.



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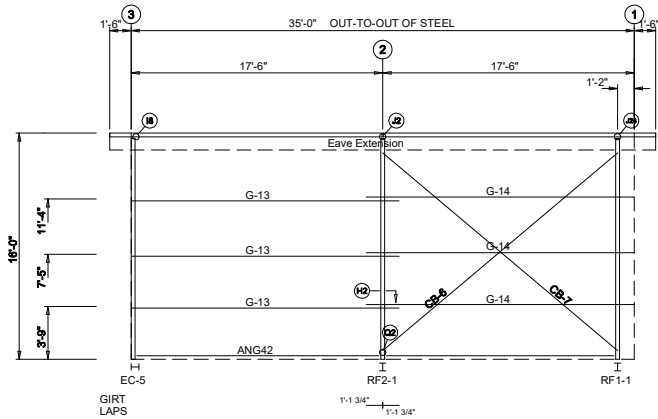
**GENERAL NOTES:**  
 Panel "Start" and "End" dimensions must be followed for the proper installation of the gable trim(s) provided.

**ROOF SHEETING PLAN**  
 PANELS: 26 Ga. R - TBD

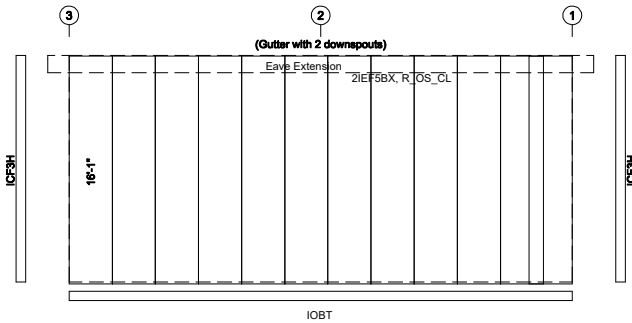
DRAWING IS NOT TO SCALE

<b>TBD</b>		<b>DRAWING STATUS</b>			
<p>FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p>FOR CONSTRUCTION: FINAL DRAWINGS.</p>	<p>CITY: TBD</p> <p>ST / PW: TBD</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 50%;">REVISION HISTORY</th> <th style="width: 50%;">DATE</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> </tr> </tbody> </table>	REVISION HISTORY	DATE		
REVISION HISTORY	DATE				
<p><b>F.O. Innisfil ON36x35x16</b></p>		<p><b>04-05-2024</b> ISSUED FOR PERMIT</p>			
<p><b>PROFESSIONAL ENGINEER</b> G. THAI 100080821 PROVINCE OF ONTARIO</p>		<p><b>04-05-2024</b> ISSUED FOR PERMIT</p>			
<p><b>36'-0" x 35'-0" x 16'-0"</b></p>		<p><b>DATE: 4/3/24</b></p>			
<p><b>REVISION: 0</b></p>		<p><b>APPD:</b></p>			
<p><b>ENG:</b></p>		<p><b>DWN:</b></p>			
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**SIDEWALL FRAMING: FRAME LINE D**



**SIDEWALL SHEETING & TRIM: FRAME LINE D**  
PANELS: 26 Ga. R - TBD

**GENERAL NOTES:**

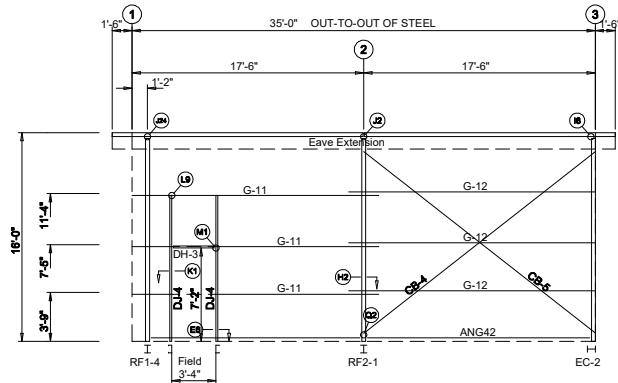
1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

**DRAWING IS NOT TO SCALE**

**TRIM COLORS**

EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.	

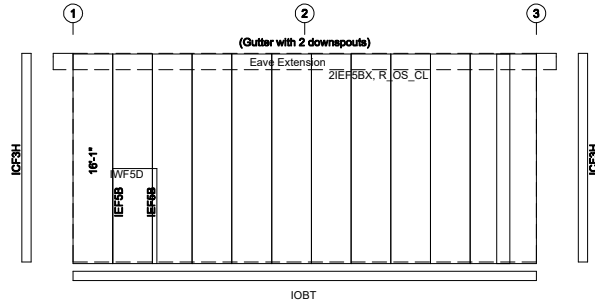
<b>TBD</b>		<b>ST / PW: TBD</b>	
<b>DRAWING STATUS</b>		<b>REVISION HISTORY</b>	
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GIRT LAPS

1'-1 3/4" 1'-1 3/4"

**SIDEWALL FRAMING: FRAME LINE A**



**SIDEWALL SHEETING & TRIM: FRAME LINE A**

PANELS: 26 Ga. R - TBD

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